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**HYDRAULIC MODEL
INVESTIGATION**

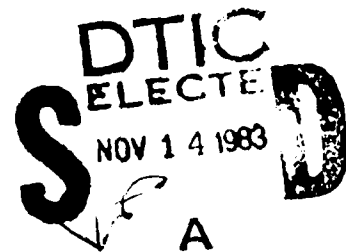
TECHNICAL REPORT NO. 194-1

Emergency Closure System and Flood Control Regulation Gate for Hiram M. Chittenden Locks at Lake Washington Ship Canal

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20. ABSTRACT (Continue on reverse side if necessary and identify by block number) -A 1:50-scale model was used to evaluate the proposed emergency closure sys- tem (ECS) for the large lock at the Lake Washington Ship Canal project. The model was also used to evaluate the acceptability of using the large lock chamber as an auxiliary spillway with the ECS functioning as a regulating gate. (continued)		

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In the emergency closure mode, maximum hydrodynamic downpull forces of 20 Kips were measured on the ECS lowering cables when the bulkheads were stacked one on top of the other (operating plan). Individual bulkheads could not be successfully placed in the flow.

The model showed that the ECS would be acceptable as a regulating gate for the large lock; however, some modifications within the large lock chamber would be required prior to the lock's satisfactory performance as an auxiliary spillway.

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PREFACE

Model studies of the Emergency Closure System (ECS) at the Lake Washington Ship Canal and Hiram M. Chittenden Locks project were authorized by North Pacific Division (NPD) on 3 April 1981, at the request of the U.S. Army Corps of Engineers, Seattle District (NPS). Studies were conducted at the Division Hydraulic Laboratory, U.S. Army Engineering Division, North Pacific, during the period May 1981 through April 1982.

The model studies were conducted by Mr. R. L. Johnson under the supervision of Mr. P. M. Smith, Director, of the Laboratory. This report was prepared by Mr. M. M. Kubo, NPS Hydraulics Section.



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CONVERSION FACTORS, U.S. CUSTOMARY TO METRIC (SI)
UNITS OF MEASUREMENT

U.S. customary units of measurement used in this report can be converted to metric (SI) units as follows:

<u>MULTIPLY</u>	<u>BY</u>	<u>TO OBTAIN</u>
Feet	0.3048	Meters
Miles	1.6093	Kilometers
Feet per Second	0.3048	Meter per Second
Cubic Feet per Second	0.02832	Cubic Meters per Second
Pounds (Mass)	0.45359	Kilograms

EMERGENCY CLOSURE SYSTEM
AND FLOOD CONTROL REGULATION GATE
FOR HIRAM M. CHITTENDEN LOCKS
AT LAKE WASHINGTON SHIP CANAL

Hydraulic Model Investigations

PART I: INTRODUCTION

The Prototype

1. Lake Washington Ship Canal (LWSC) and Hiram M. Chittenden Locks, located in Seattle, Washington (figure 1), connects freshwater Lake Washington through Lake Union to saltwater Shilshole Bay and Puget Sound (plate 1). The locks are located at the entrance to Salmon Bay. General arrangement of the project facilities and appurtenant structures are as shown on plate 2.

Design Considerations

2. The proposed emergency closure system (ECS) is to be installed at the entrance to the large lock. The system consists of a series of bulkheads which would be stacked one on top of the other and lowered into the lock chamber by carriages located in slots on the lock walls. During emergency closure conditions, flow would occur only under the bottom bulkhead.

3. Probable maximum flood (PMF) analysis indicates that present project outflow capability is inadequate and additional outflow capability will be required to control Lake Washington to presently accepted levels during passage of floods approaching standard project flood up to PMF magnitude. The use of the large lock chamber as an auxiliary spillway with the ECS functioning as a regulating gate would be considered as a viable alternative for providing additional discharge capacity.

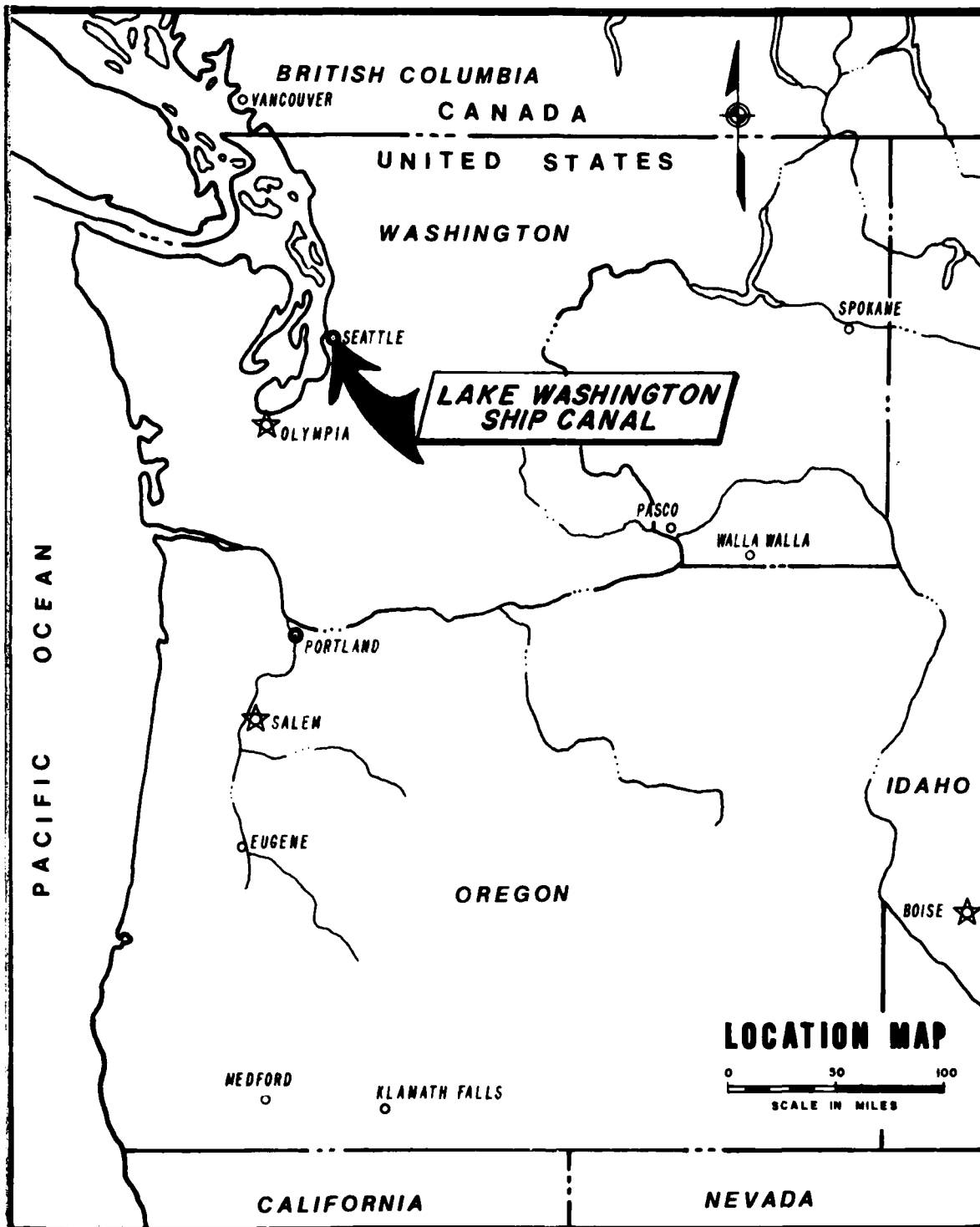


Figure 1

Purpose and Need for Model Study

4. The purpose of the model study was to define the feasibility of the ECS and verify its competency as a regulating gate. The model study was required to provide hydrodynamic loading, flow conditions, and hydraulic characteristics associated with emergency closure as well as using the ECS as a regulating gate insuring the compatibility for dual purpose.

PART II: THE MODEL

Description

5. The proposed ECS, constructed to a scale of 1 to 50, consisted of seven bulkheads to be lowered in new slots in the upstream end of the lock. The initial bulkhead would be placed on cable-suspended lowering carriages in the slots. The remaining bulkheads would be stacked in the slots on top of the first one as the entire system was being lowered into the flow such that flow would be only under the stacked bulkheads. With the lock used as an auxiliary spillway, the closure bulkheads would be the control gate and the lock would function essentially as an energy dissipator.

6. The model included the 80-foot by 1,100-foot lock structure and 400 feet of approach upstream and downstream (photographs 1 and 2 and plates 3 and 4). The filling and emptying system of the lock was not reproduced. The lock gates were simulated in the open position by smooth walls in the model. The saltwater barrier (photograph 3) was simulated using only the minimum ballast required to maintain a down position which resulted in maximum sensitivity to flutter or rise with small pressure fluctuations. Piezometers for measuring pressure were placed in key locations (plates 3 and 4 and table A). The bottom bulkhead was reproduced in full detail both in size and shape of individual members and mass weight (plate 5) while the remaining bulkheads were reproduced only in general shape; however, they were reproduced to simulate correct mass (photograph 4). The three bottom bulkheads weighed 52.8 kips each, and the weight of the top four were 43.9 kips each. The bulkheads were lowered in a harness (a slot that moved with the bulkheads) to keep them free of sliding friction (photograph 5) to insure that hydraulic loading was isolated from friction loading. The lowering speed was 2 feet per minute (prototype). Forces on the cables were measured with SR-4 strain gages on a force ring and recorded to the nearest kip oscillographically.

Interpretation of Test Results

7. The following accepted equations of hydraulic similitude, based on Froudian relationship in which gravity is the dominant force, were used to express the mathematical relations between the dimensions and hydraulic quantities of the model and the prototype.

<u>Dimension</u>	<u>Ratio</u>	<u>Scale Relationship</u>
Length	L_r	1:50
Area	$A_r = L_r^2$	1:2,500
Velocity	$V_r = L_r^{1/2}$	1:7.071
Time	$T_r = L_r^{1/2}$	1:7.071
Discharge	$Q_r = L_r^{5/2}$	1:17,667.67
Roughness	$N_r = L_r^{1/6}$	1:1.9194
Force	$F_r = L_r^3$	1:125,000
Weight	$W_r = L_r^3$	1:125,000

PART III: TESTS AND RESULTS

Emergency Closure System Mode

8. All tests were conducted with forebay pool el 22 feet, the maximum authorized elevation of Lake Union and Lake Washington. Tailwater was varied the full normal tide cycle from el -2 to el 12 feet.

9. The hydrodynamically induced downpull forces measured on the lowering cables as the bulkheads were placed for emergency closure are listed in table B. The forces were essentially the same with all tailwater levels. Control of the flow beneath the bulkheads remained at the sharp upstream bottom edge of the bottom bulkhead. Flow sprang free at that edge and did not impinge on the bottom members even at small submerged openings. The downpull forces gradually increased during lowering to a maximum of 20 kips at approximately 1 foot of opening. Downpull forces listed in table B were observed with the bulkheads continuously lowered at 2 feet per minute. When the bulkheads were placed to simulate prototype placement (lowering one to the point of near overflow, placing another on top, lowering again to the point of near overflow, and placing the others in like manner), the forces were the same as occurred during placement under continuous lowering during both the static and dynamic operating mode (table C). When bulkheads were held static in the flow, the forces remained the same as those developed during continuous lowering of the bulkheads as they passed through that position. The bulkheads were supported by the cable-suspended carriages attached to the bottom bulkhead and were unrestrained by friction, which permitted them maximum freedom to vibrate. No vibration occurred during lowering or during suspension at partial openings.

10. Hydraulic loading was also monitored with the bottom bulkhead modified by placing a solid plate over the upstream half of the bottom of the bottom bulkhead (photograph 6). With this bulkhead geometry, downpull forces during lowering were the same as those without the plate attached (table C). No vibration occurred with the partially plated bottom.

11. Individual bulkheads could not be placed successfully in the flow. An individual bulkhead could be lowered deep into the flow but would jam in the slots before seating. Cable forces measured during two attempts to place a single bulkhead are listed in table C. With flow over and under the bulkhead, downpull forces were as high as 51 kips. At an opening of 12 to 15 feet, the bulkhead began to be misaligned in the slots and at an opening of approximately 5 feet became jammed.

Regulating Gate Mode

12. Water surface profiles, pressures, and velocities in the lock were measured with tailwater el -2, 5, and 12 feet with free flow and bulkhead openings of 28, 23, 18, 13, 8, and 3 feet. These data for low and high tailwater and various bulkhead openings are shown on plates 6 to 21. Flow conditions with free flow and openings of 23, 13, and 3 feet are shown in photographs 7 to 18.

13. The highest discharge and velocities occurred during free flow conditions with tailwater el -2 feet. Flow passed through critical depth (22.8 feet) at the upstream operating gate sill and was supercritical the full length of the lock chamber (plate 6 and photograph 7). A hydraulic jump occurred over the downstream gate sills. Maximum velocity along the floor and walls of the lock was 46 feet per second (fps) (plate 7). Maximum velocity on the channel bottom downstream from the lock chamber was 19 fps. All pressures were positive (plate 6).

14. The maximum local velocity and lowest pressure occurred with tailwater elevation -2 feet and a bulkhead opening of 8 feet (plates 10 and 11). Velocity on the floor just downstream from the bulkheads was 50 fps. Pressure just downstream from the existing closure system floor recess, (piezometer 7) was -17 feet of water. Pressure on the wall just downstream from the bulkhead slots was -6 feet (piezometer 4). The average pressure of -17 feet indicates a potential for cavitation exists in proximity of the floor recess during extreme conditions. Pressures at most measuring points fluctuated 1 to 2 feet of water with a maximum fluctuation of approximately 5 feet.

15. With the closure floor recess filled to the adjacent floor level, elevation -16 feet (photograph 19), the lowest pressures still

occurred with the 8-foot opening but were 6 and 9 feet of water at piezometers 7 and 4, respectively (tables D and E). The minimum pressure, which occurred downstream at piezometer A, was -7 feet of water — an acceptable level.

16. Piezometers were located on the right wall of the lock chamber near the upstream edge of the lock's four sets of miter gates. The piezometers were used to evaluate the effect of large discharges on the miter gates in an open condition to simulate operating the lock chamber as an auxiliary spillway. Pressures at those locations were approximately equal to the water depth in the chamber except at the two upstream miter gates during conditions of low tailwater and high velocity near the gates. Under the conditions, pressures were 2 to 10 feet less than the water depth (plates 8 and 22). These data indicate that at the higher velocity conditions, pressure in the recess behind the miter gates could be greater than pressures on the front face of the gate, thus tending to force the gates into the flow.

17. With almost all conditions observed, the saltwater barrier (simulated with minimum ballast) fluttered if not restrained. Sloped, low sills on the downstream edge of the barrier were tested first at the two sides and then across the full width of the barrier in an attempt to prevent flutter; however, neither plan was successful. A deflector which diverted flow upward at the downstream face of the sill under the barrier away from the overhanging edge of the barrier was also ineffective. With a sharp-edged sill placed immediately upstream from the barrier to create a reduced pressure at the upstream edge and a sill placed under the downstream edge to minimize return flow under the barrier (plate 23), the barrier remained down and stable under all flow conditions. Neither sill alone was effective with all flow conditions. The downstream sill was required the full width of the lock to block return flow. The upstream sill must be the full 2-feet height of the barrier and a minimum of 2 inches upstream from the barrier to allow for barrier rotation into the raised position. The sill may be as much as 6 inches upstream and still be effective in lowering pressures beneath the barrier.

Discharge Rating

18. Discharge capacity of the lock free of tailwater effect with various forebay pool levels and bulkhead openings is shown on plate 24. The upper limits of effect of various tailwater levels are also shown. Below those limits the tailwater would reduce the discharge that would pass through the lock. The discharge with free flow conditions and three tailwater levels is shown on plate 25. The maximum discharge capacity of the lock with forebay el 22 feet is 48,700 cfs (free flow).

PART IV: SUMMARY

19. A 1:50 scale model was used to verify the design of an ECS and evaluate the same system for compatibility as a flood control regulating gate for Hiram M. Chittenden Locks at LWSC.

20. The bulkhead closure system proved acceptable both for emergency closure and as a regulating gate. Hydraulic downpull on the bulkheads was low (20 kips maximum), and no uplift occurred to prevent seating. The bulkheads did not vibrate during lowering or when suspended in the flow. The lock chamber functioned well as a spillway but would require some modifications, i.e., filling of the existing closure recess on the chamber floor, restraint of the miter gates against closure, and modification of the saltwater barrier prior to using as an auxiliary spillway.

TABLE A
PIEZOMETER LOCATIONS

Piezometer Number	Distance in Feet		Elevation
	From Downstream End of Lock	From Right (North) Wall	
1	E 1085.50	-3.23	-15
2	E 1083.40	0	-15
3	E 1082.15	0	-14
4	E 1080.65	0	-15
5	E 1085.50	45.00	-16
6	E 1075.50	45.00	-18
7	E 1059.50	45.00	-16
8	E 1042.50	0	-19
9	E 1042.00	45.00	-20
A	E 1007.50	45.00	-16
B	E 1001.00	60.00	-16
10	E 972.50	0	-20
11	E 972.00	45.00	-21
12	E 937.50	45.00	-16
13	E 919.89	45.00	-29
14	E 722.45	45.00	-29
15	E 522.00	0	-33
16	E 522.00	45.00	-34
17	E 487.50	45.00	-29
18	E 320.63	45.00	-29
19	E 147.00	0	-33
20	E 13.00	0	-33
21	E 13.00	45.00	-34

NOTE: Piezometer locations shown on plates 3 and 4.

TABLE B
HYDRODYNAMIC DOWNPULL FORCES ON BULKHEADS
Forebay El 22
Original Design Bulkheads

Bulkhead Opening in Feet	Tailwater Elevation											
	-2				1				5			
	9											
	12											
Force in Kips												
	Run 1	Run 2	Run 3	Run 4	Run 1	Run 2	Run 1	Run 2	Run 1	Run 2	Run 1	Run 2
32.0	0	0	0	0	0	0	0	0	0	0	0	0
31.0	1	0	1	1	1	1	1	1	0	1	1	1
26.0	2	2	3	2	3	2	2	2	1	2	2	2
21.0	3	4	5	3	4	3	3	4	2	4	4	3
16.0	4	5	7	5	6	4	5	5	3	5	6	4
11.0	5	7	9	6	8	6	7	7	4	7	8	5
6.0	11	13	14	13	14	11	14	13	10	12	12	17
1.0	16	20	19	19	20	17	20	19	16	17	16	20
0.1	16	19	19	19	19	16	19	20	14	16	16	20
0	0	0	0	0	0	0	0	0	0	0	0	0

NOTE: Bulkheads lowered at 2 fpm without stopping.

TABLE B

TABLE C
HYDRODYNAMIC DOWNPULL FORCES ON BULKHEADS
Forebay E1 22

Bulkhead Opening in Feet	Original Design Bulkheads		Upstream Half of Bottom of Bottom Bulkhead Plated	
	Single Bulkhead Lowered		Bulkheads Lowered Without Stopping	
	Bulkheads Added One at a Time			
	Tailwater Elevation			
	5	12	-2	12
Force in Kips				
	Run 1	Run 1	Run 2	Run 2
32.0	0	0	0	0
31.0	1	9	1	1
26.0	2	31	2	2
21.0	4	42	4	4
16.0	5	41	6	6
11.0	7	11	8	7
6.0	12	-19	14	13
1.0	18	-26	20	18
0.1	18	-26	19	18
0	0	-26	0	0

TABLE C

TABLE D

PRESSURES

Existing Closure Floor Recess Filled
Forebay El 22, Tailwater El -2

Piezometer Number	Lock Discharge in CFS						
	48,700	45,500	41,500	35,780	27,860	18,330	6,430
	Bulkhead Opening in Feet						
	Free Flow	28	23	18	13	8	3
	Pressure in Feet of Water						
1	25	29	32	33	30	21	24
2	25	21	19	27	13	8	5
3	24	21	17	25	12	7	4
4	25	23	23	22	16	9	6
5	25	21	18	16	16	15	4
7	28	21	17	15	10	6	7
8	30	22	17	13	9	4	12
9	32	22	18	14	9	6	13
A	29	20	15	10	1	-7	9
B	28	21	17	13	5	0	10
10	27	26	23	19	16	12	17
11	27	26	23	20	17	13	18
12	17	21	17	14	10	5	13
13	21	21	19	17	15	22	27

NOTE: Piezometer locations shown on plates 3 and 4.

TABLE D

TABLE E

PRESSURES

Existing Closure Floor Recess Filled
Forebay El 22, Tailwater El 5

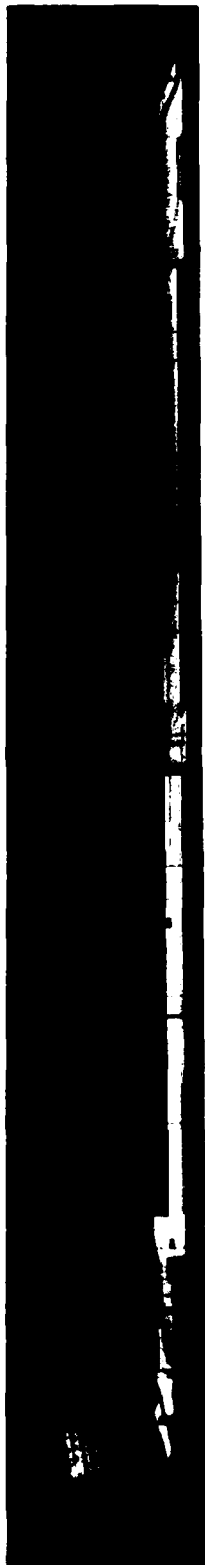
Piezometer Number	Lock Discharge in CFS						
	48,700	45,500	41,500	35,780	27,860	18,330	5,360
	Bulkhead Opening in Feet						
	Free Flow	28	23	18	13	8	3
	Pressure in Feet of Water						
1	25	29	32	33	30	21	28
2	25	21	18	17	13	9	15
3	24	20	17	15	12	7	13
4	25	23	23	21	16	9	17
5	25	20	18	16	16	15	18
7	28	21	17	15	10	6	18
8	30	22	18	13	9	14	23
9	32	22	18	13	10	13	23
A	29	20	15	9	2	12	20
B	28	21	17	13	6	15	20
10	27	26	23	20	17	22	25
11	27	26	23	21	19	22	26
12	17	21	18	16	14	18	21
13	21	22	22	24	29	32	34

NOTE: Piezometer locations shown on plates 3 and 4.

TABLE E



Photograph 1
Lake Washington Ship Canal Lock Model
Looking upstream



Upstream

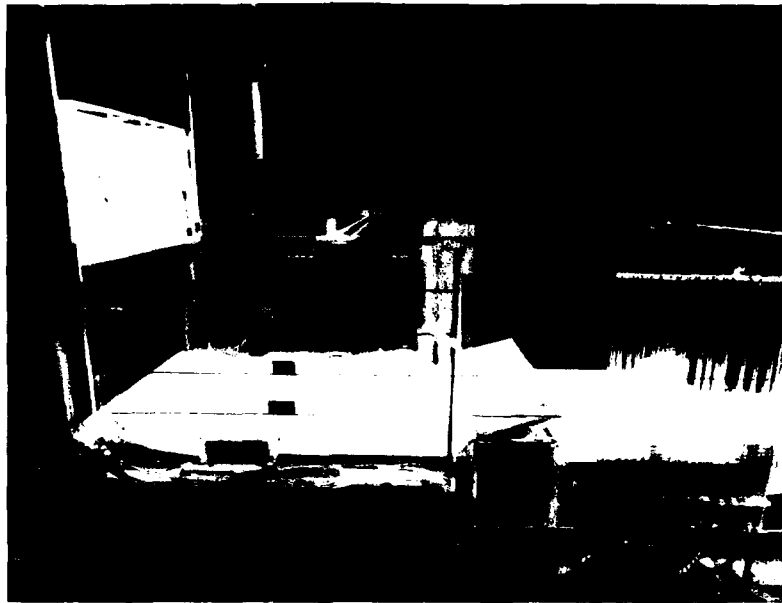
Closure bulkheads



Saltwater barrier

Photograph 2

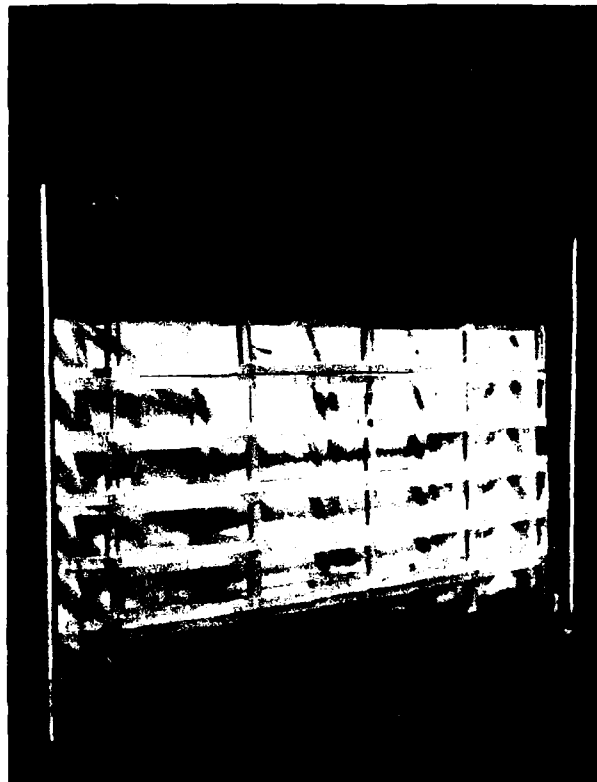
Lock chamber



Closure recess

Saltwater barrier
(partially raised)

Photograph 3
Existing lock floor



Photograph 4
Emergency closure bulkheads stacked in model lowering harness
Downstream face



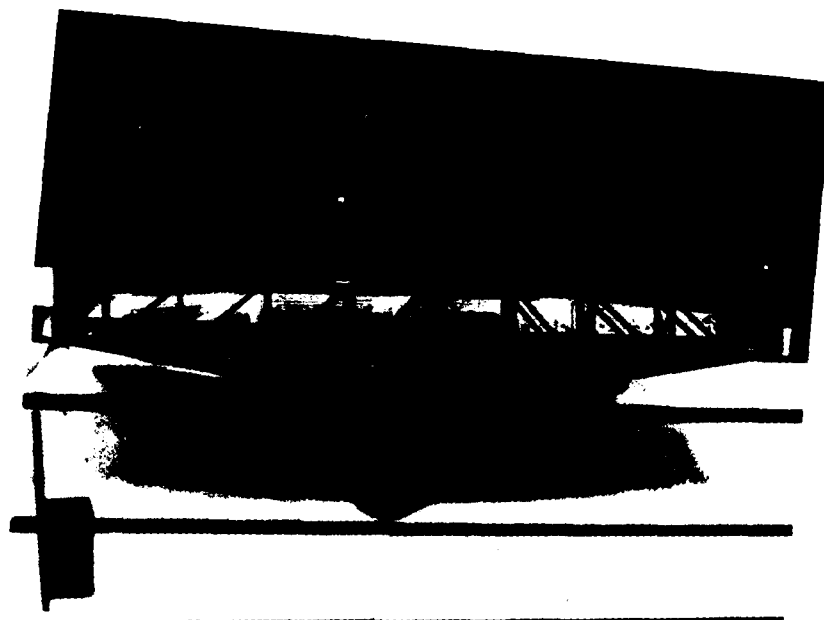
Downstream face



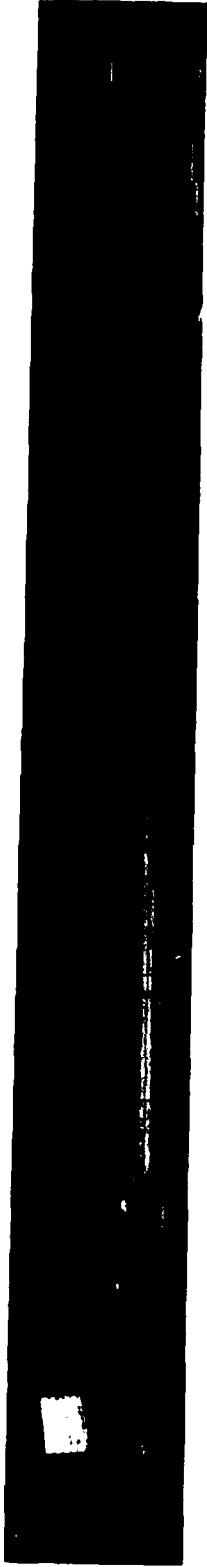
Upstream face

Photograph 5

Emergency closure bulkheads stacked in model lowering harness
suspended from lowering mechanism with force measuring ring

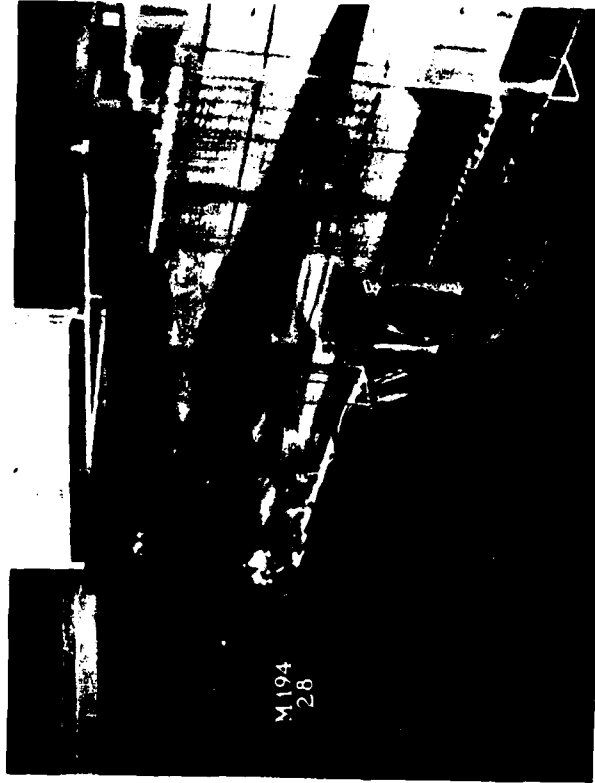


Photograph 6
Bottom of bottom bulkhead with flat plate on upstream half



Closure bulkheads

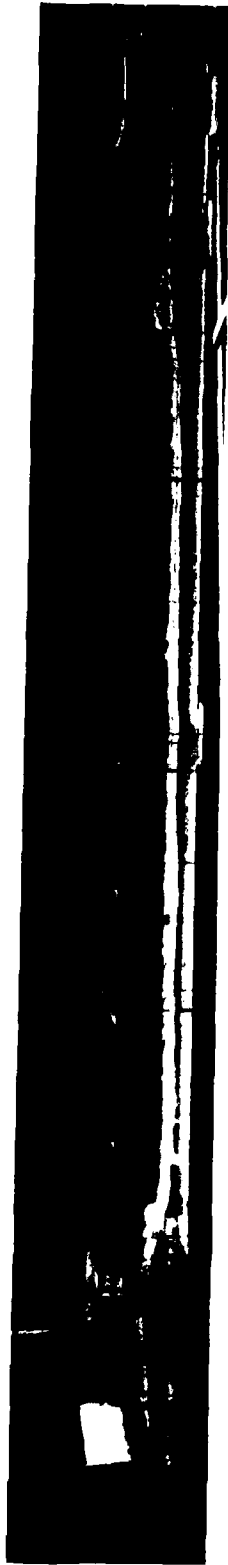
Downstream gate sills



Closure area and upstream gate sills

Photograph 7

Flow through lock chamber
Forebay el 22, tailwater el -2, free flow, discharge 48,700 cfs



Closure bulkheads

Downstream gate sills



Closure area and upstream gate sills

Photograph 8

Flow through lock chamber
Forebay el 22, tailwater el -2, bulkhead opening 23 ft, discharge 41,500 cfs



Closure bulkheads

Downstream gate sills

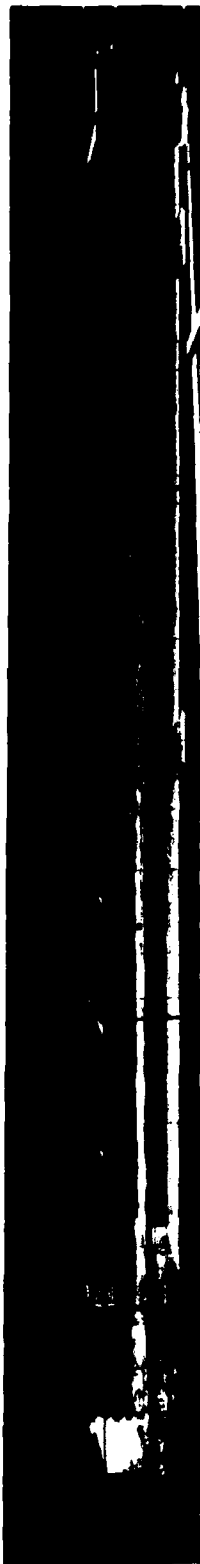


Closure area and upstream gate sills

Photograph 9

Flow through lock chamber

Forebay el 22, tailwater el -2, bulkhead opening 13 ft, discharge 27,860 cfs



Closure bulkheads

Downstream gate sills



Closure area and upstream gate sills

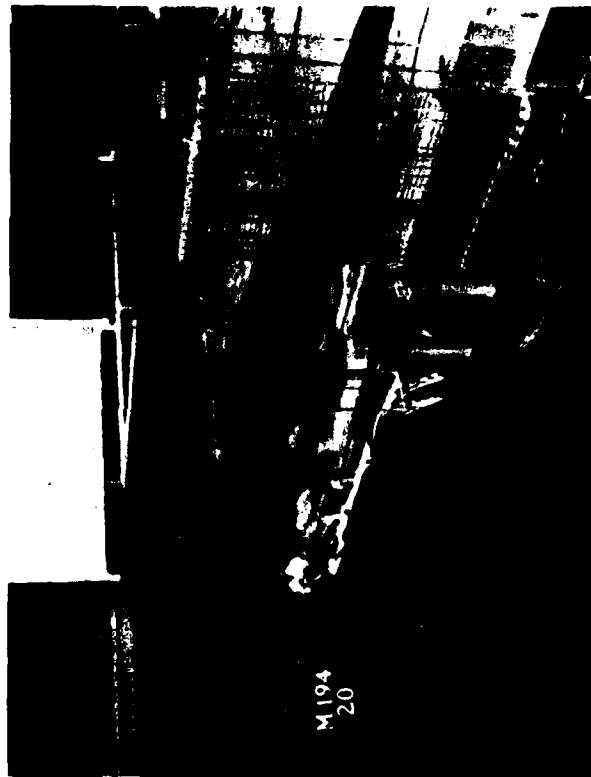
Photograph 10

Flow through lock chamber
Forebay el 22, tailwater el -2, bulkhead opening 3 ft, discharge 6,430 cfs



Closure bulkheads

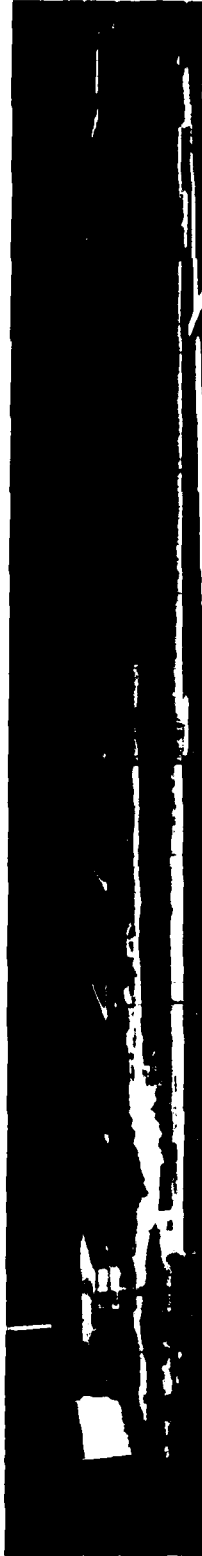
Downstream gate sills



Closure area and upstream gate sills

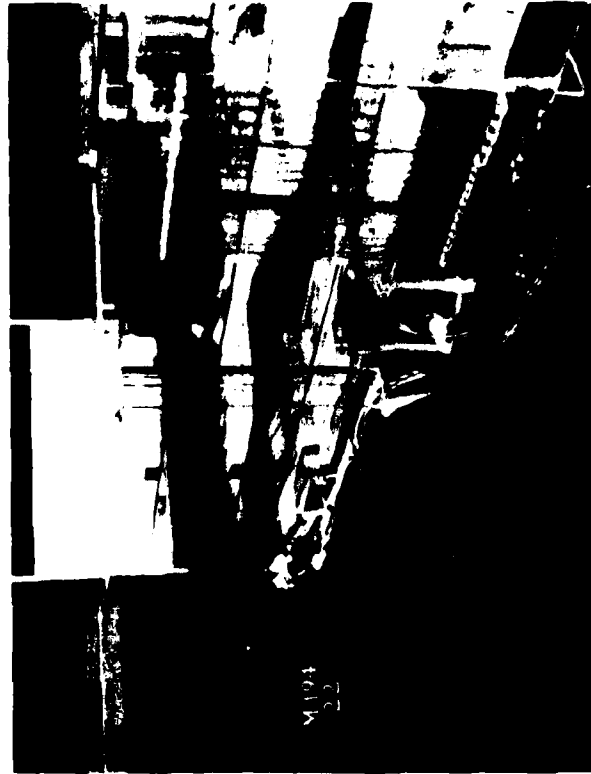
Photograph 11

Flow through lock chamber
Forebay el 22, tailwater el 5, free flow, discharge 48,700 cfs



Closure bulkheads

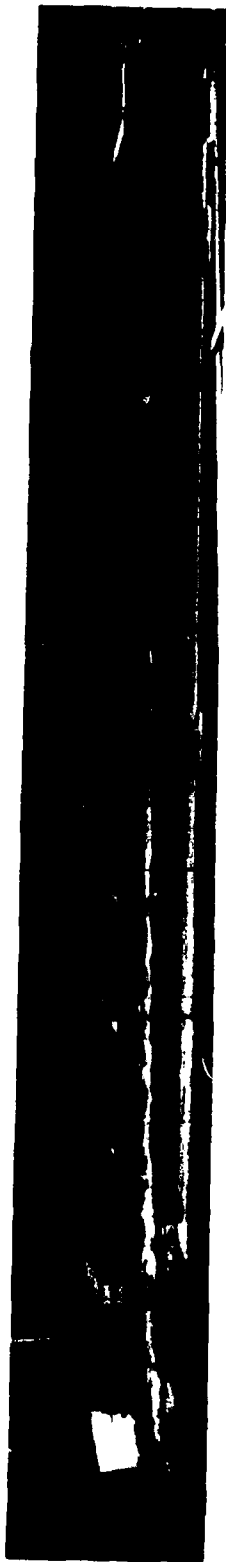
Downstream gate sills



Closure area and upstream gate sills

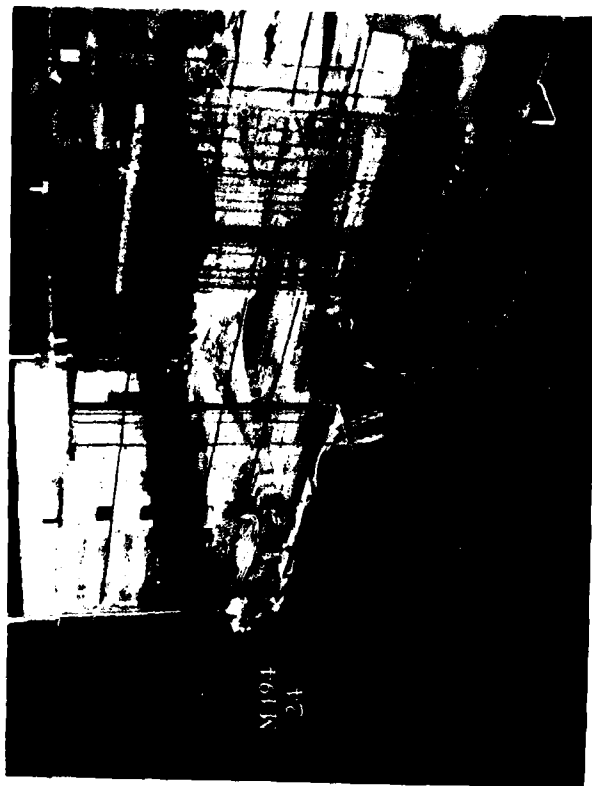
Photograph 12

Forebay el 22, tailwater el 5, bulkhead opening 23 ft, discharge 41,500 cfs



Closure bulkheads

Downstream gate sills

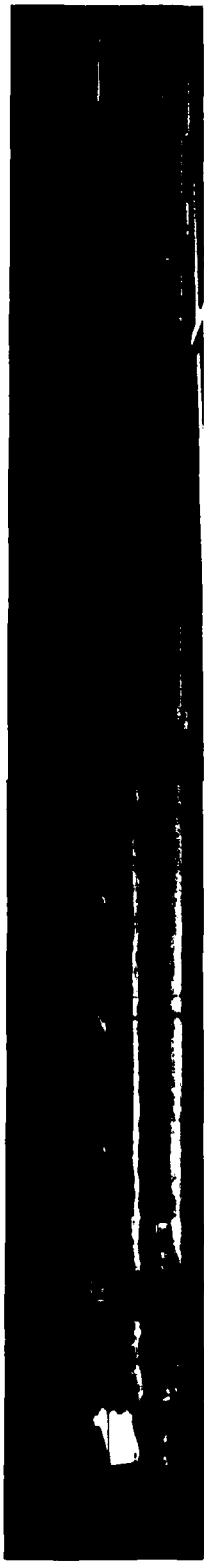


Closure area and upstream gate sills

Photograph 13

Flow through lock chamber

Forebay el 22, tailwater el 5, bulkhead opening 13 ft, discharge 27,860 cfs



Closure Bulkheads

Downstream gate sills

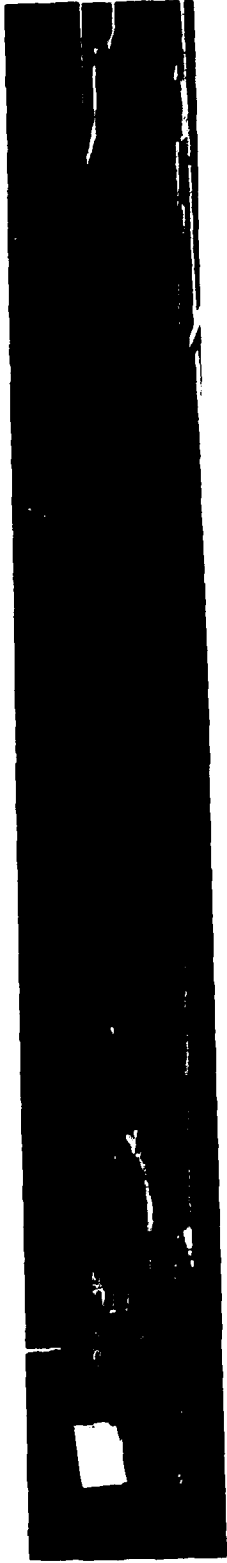


Closure area and upstream gate sills

Photograph 14

Flow through lock chamber

Forebay el 22, tailwater el 5, bulkhead opening 3 ft, discharge 5,360 cfs



Closure bulkheads

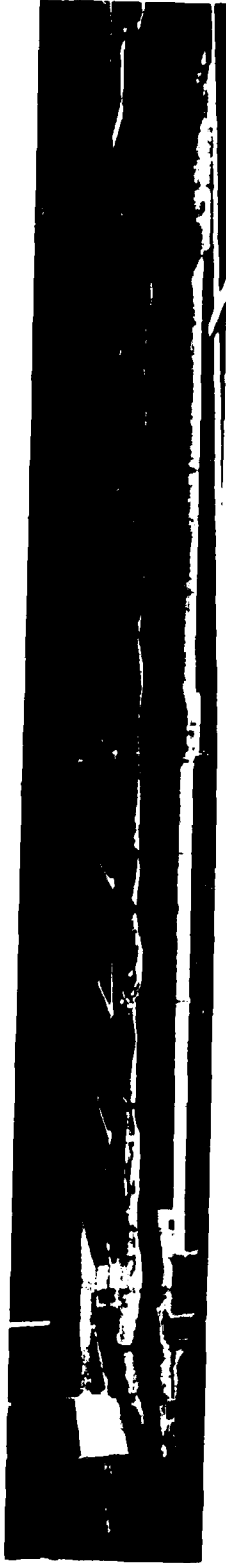
Downstream gate sills



Closure area and upstream gate sills

Photograph 15

Flow through lock chamber
Forebay el 22, tailwater el 12, free flow, discharge 48,700 cfs



Closure bulkheads

Downstream gate sills



Closure area and upstream gate sills

Photograph 16

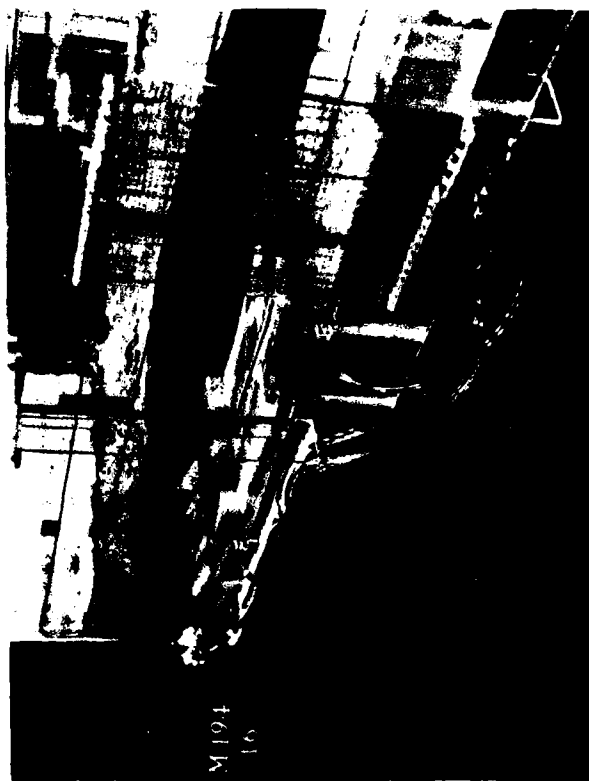
Flow through lock chamber

Forebay el 22, tailwater el 12, bulkhead opening 23 ft, discharge 41,500 cfs



Closure bulkheads

Downstream gate sills

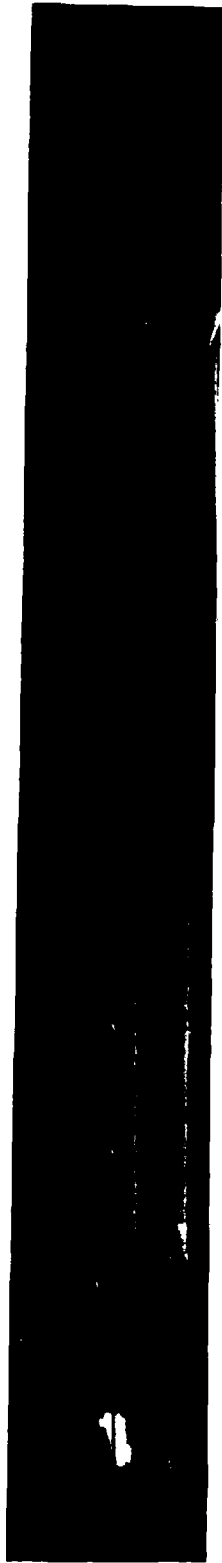


Closure area and upstream gate sills

Photograph 17

Flow through lock chamber

Forebay el 22, tailwater el 12, bulkhead opening 13 ft, discharge 21,500 cfs



Closure bulkheads

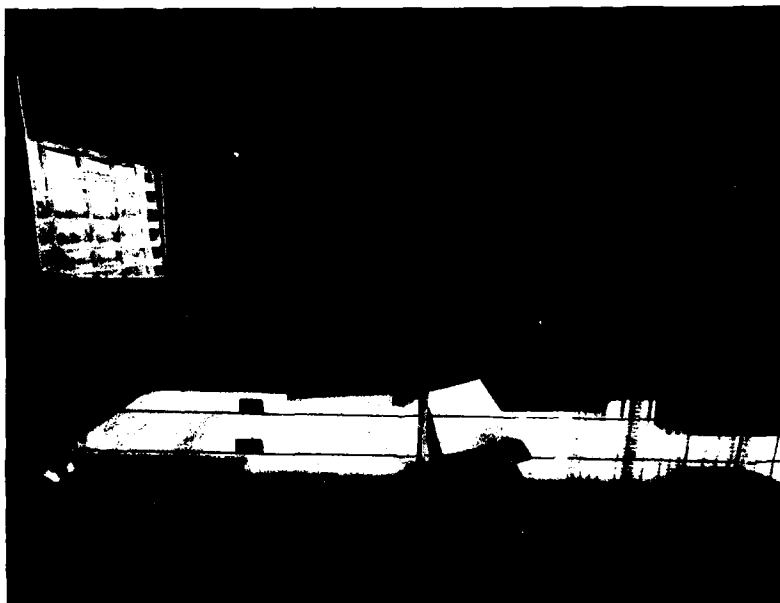
Downstream gate sills



Closure area and upstream gate sills

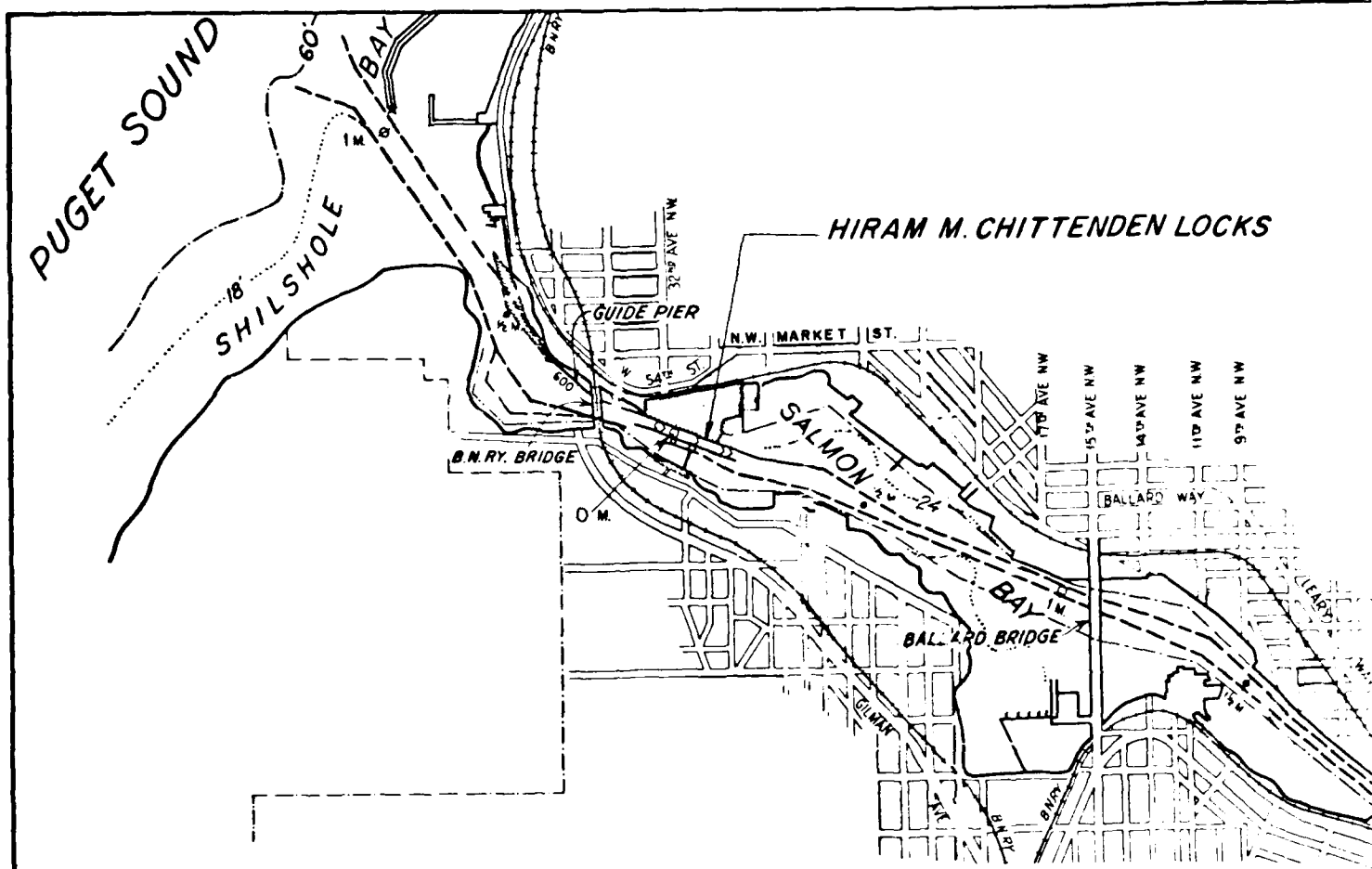
Photograph 18

Flow through lock chamber
Forebay el 22, tailwater el 12, bulkhead opening 3 ft, discharge 4,100 cfs

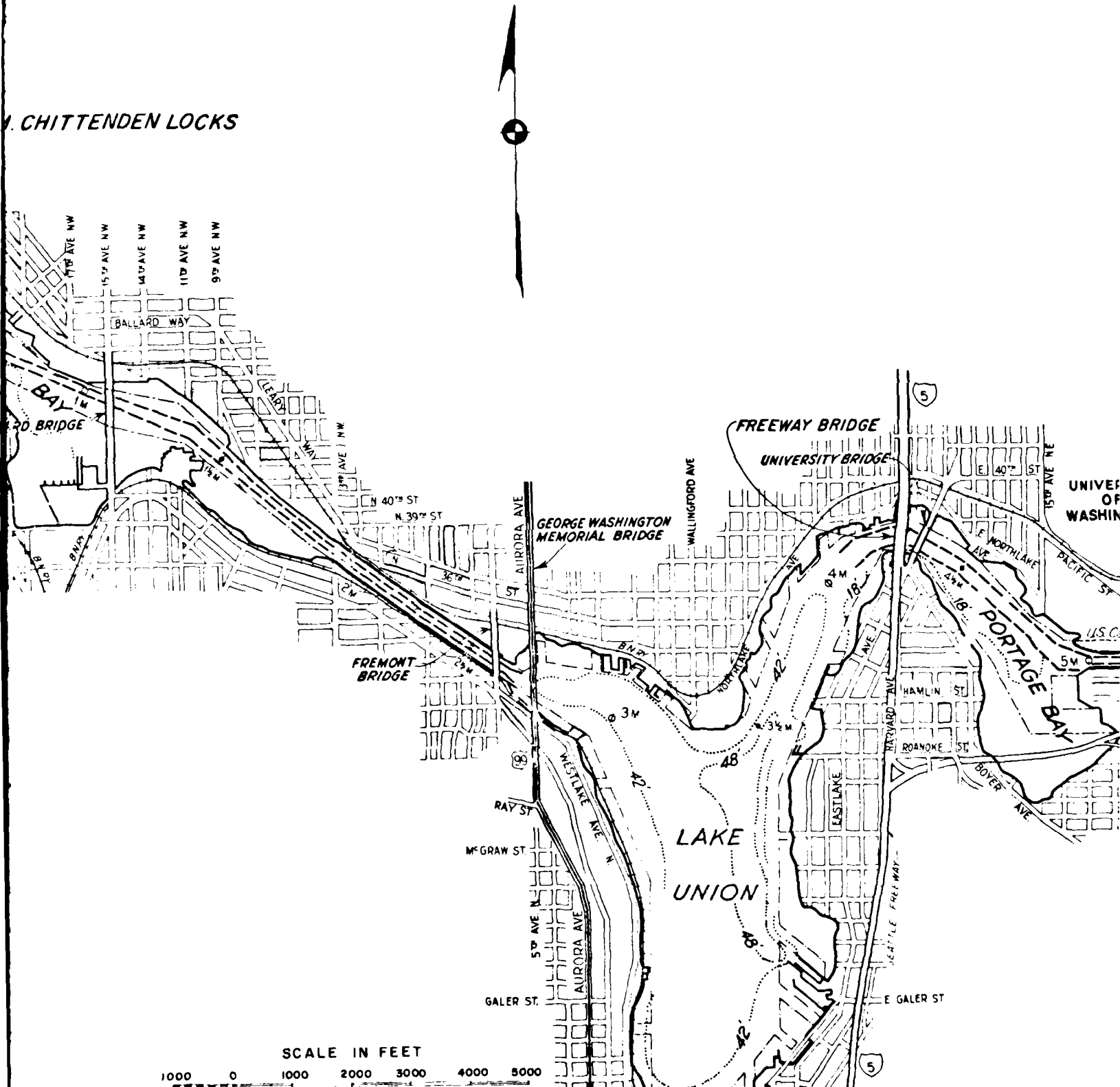


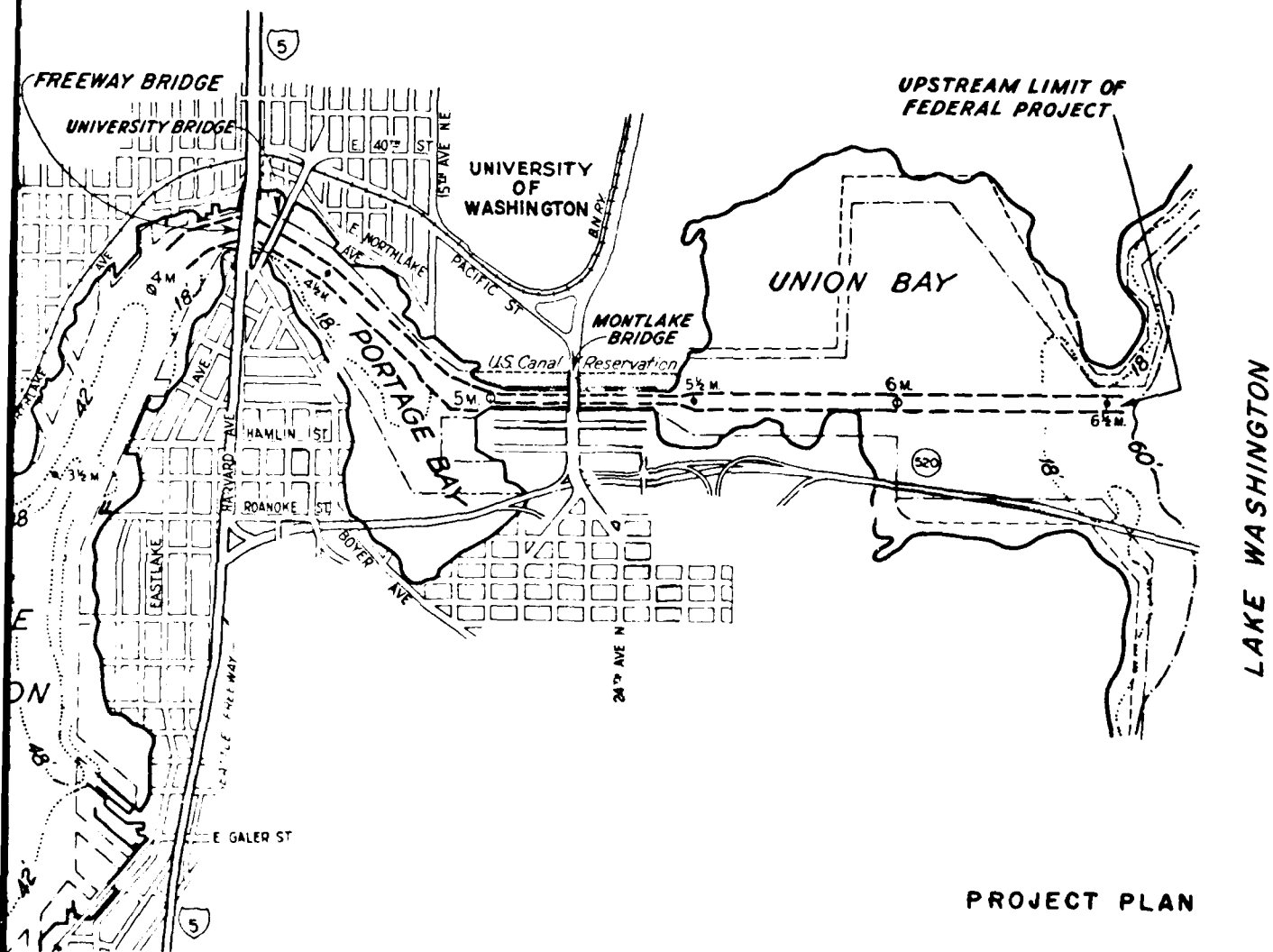
Photograph 19

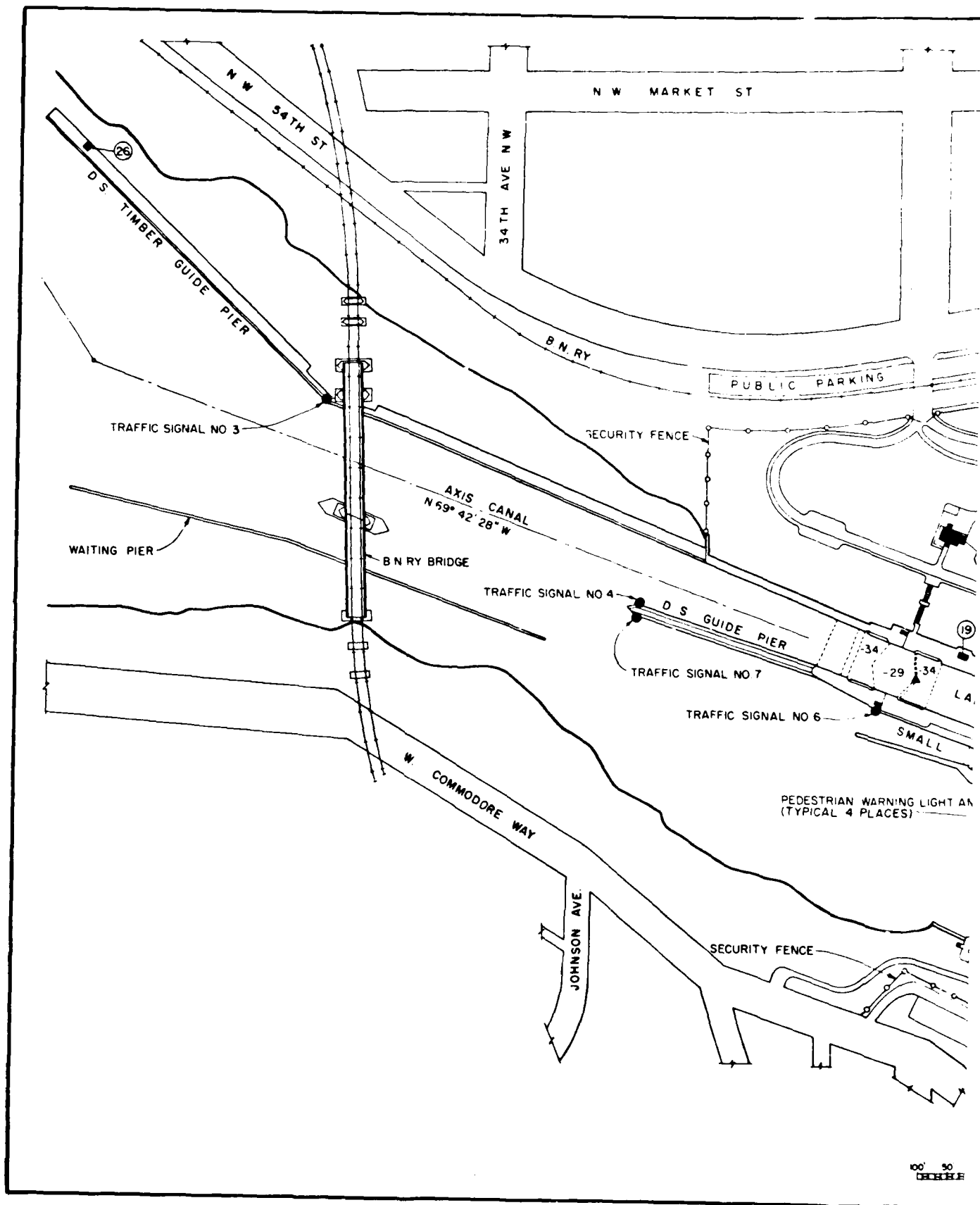
Proposed modification in gate closure recess
on floor of lock chamber

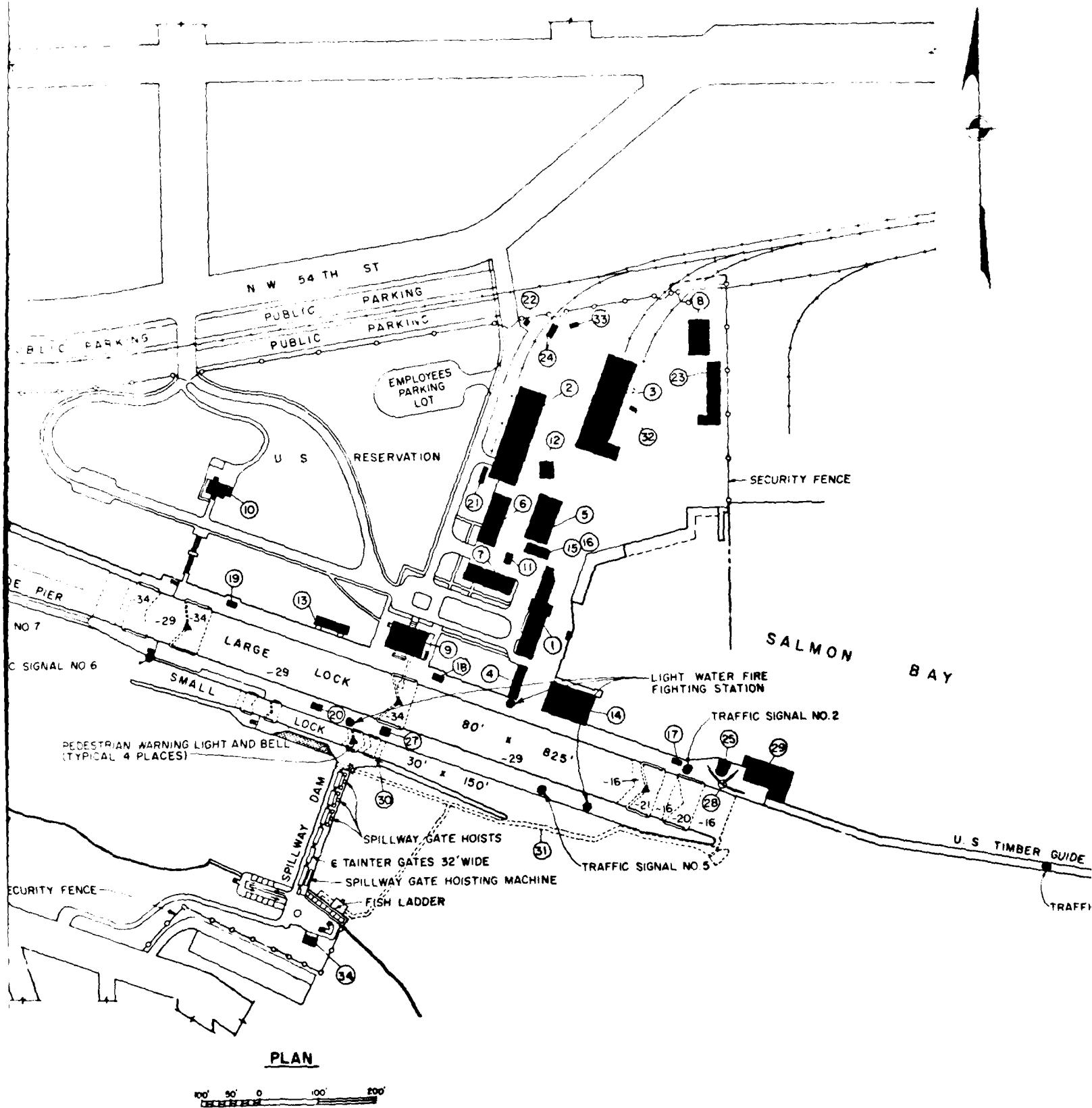


CHITTENDEN LOCKS



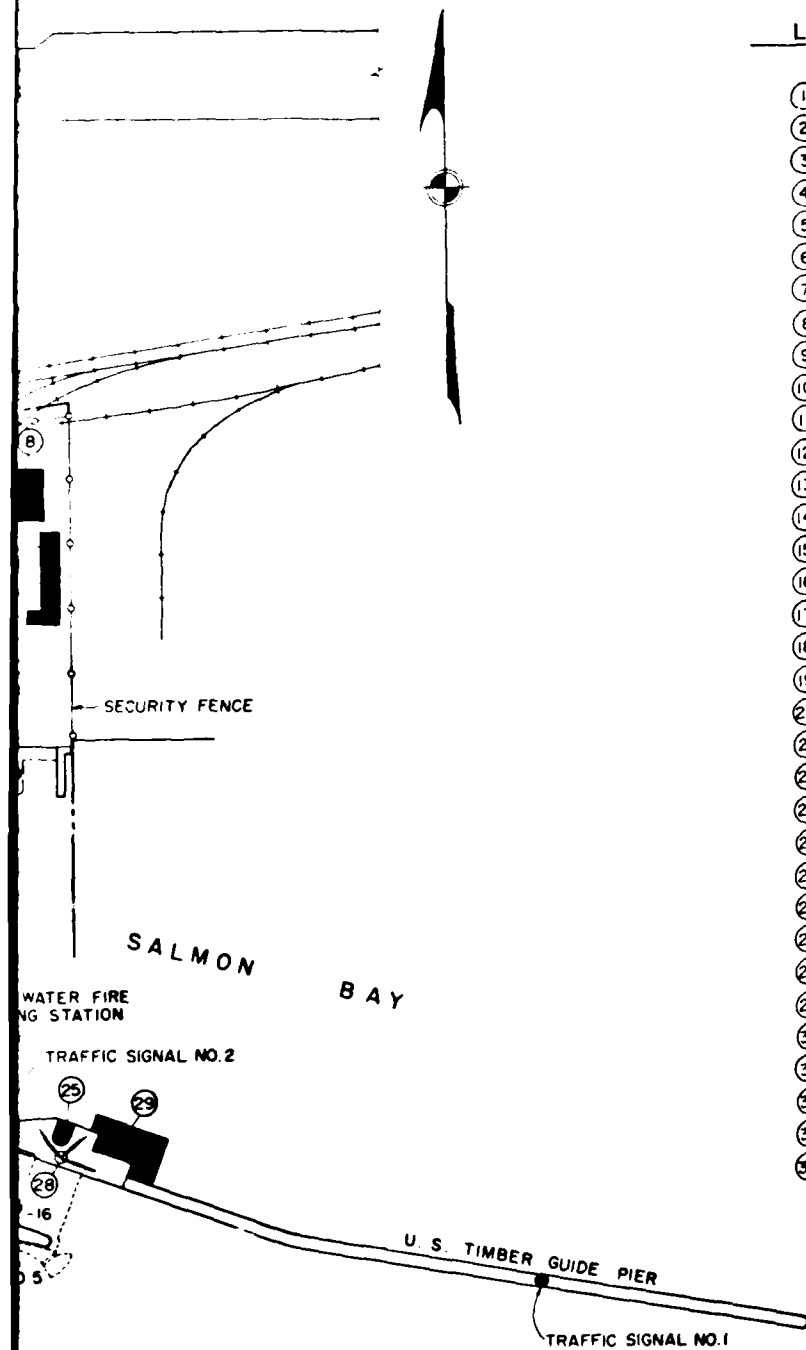






LIST OF BUILDINGS OR STRUCTURES

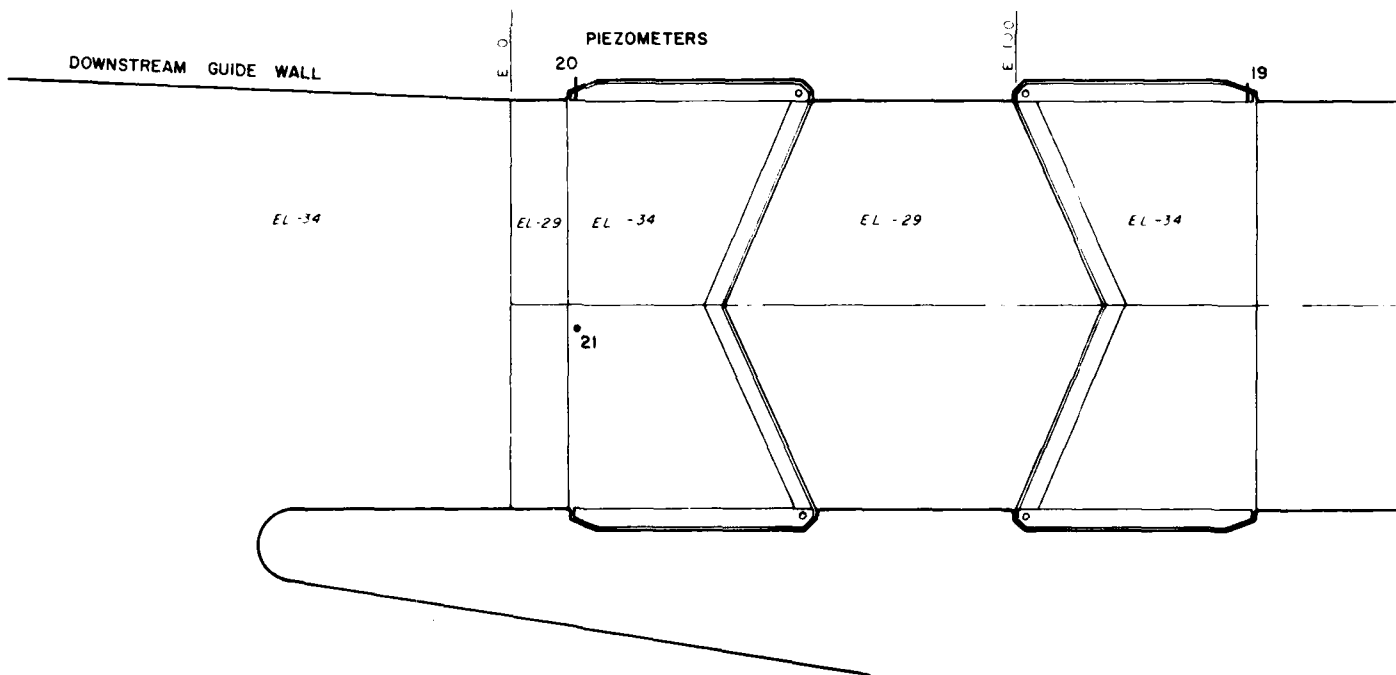
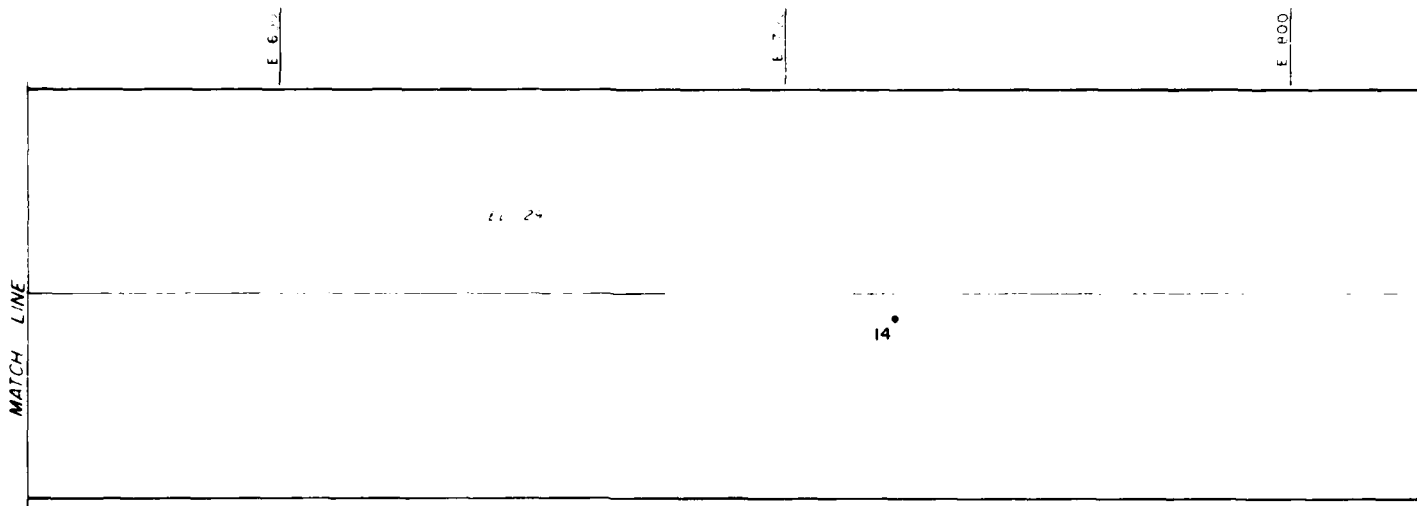
- (1) Office and Shop Building
- (2) Warehouse No 2
- (3) Garage
- (4) Mechanics Shop
- (5) Steel Shop
- (6) Carpenter and Blacksmith Shops
- (7) Machine Shop
- (8) Quonset Hut
- (9) Administration Building
- (10) District Engineer's Residence
- (11) Gas and Oil Building
- (12) Transformer House
- (13) Public Comfort Station
- (14) Bathhouse
- (15) Greenhouse
- (16) Greenhouse
- (17) Operating House No 1
- (18) Operating House No 2
- (19) Operating House No 3
- (20) Operating House No 4
- (21) Wood Storage Shed
- (22) Gatehouse
- (23) Open Storage Shed
- (24) Propane Storage
- (25) Emergency Dam Hoist House
- (26) T V Camera Pylon
- (27) Control Tower
- (28) 75-Ton Crane
- (29) Emergency Dam Storage
- (30) Emergency Dam and Derrick
- (31) Salt Water Siphon
- (32) Gas Pump
- (33) Transformer Station
- (34) Public Comfort Station



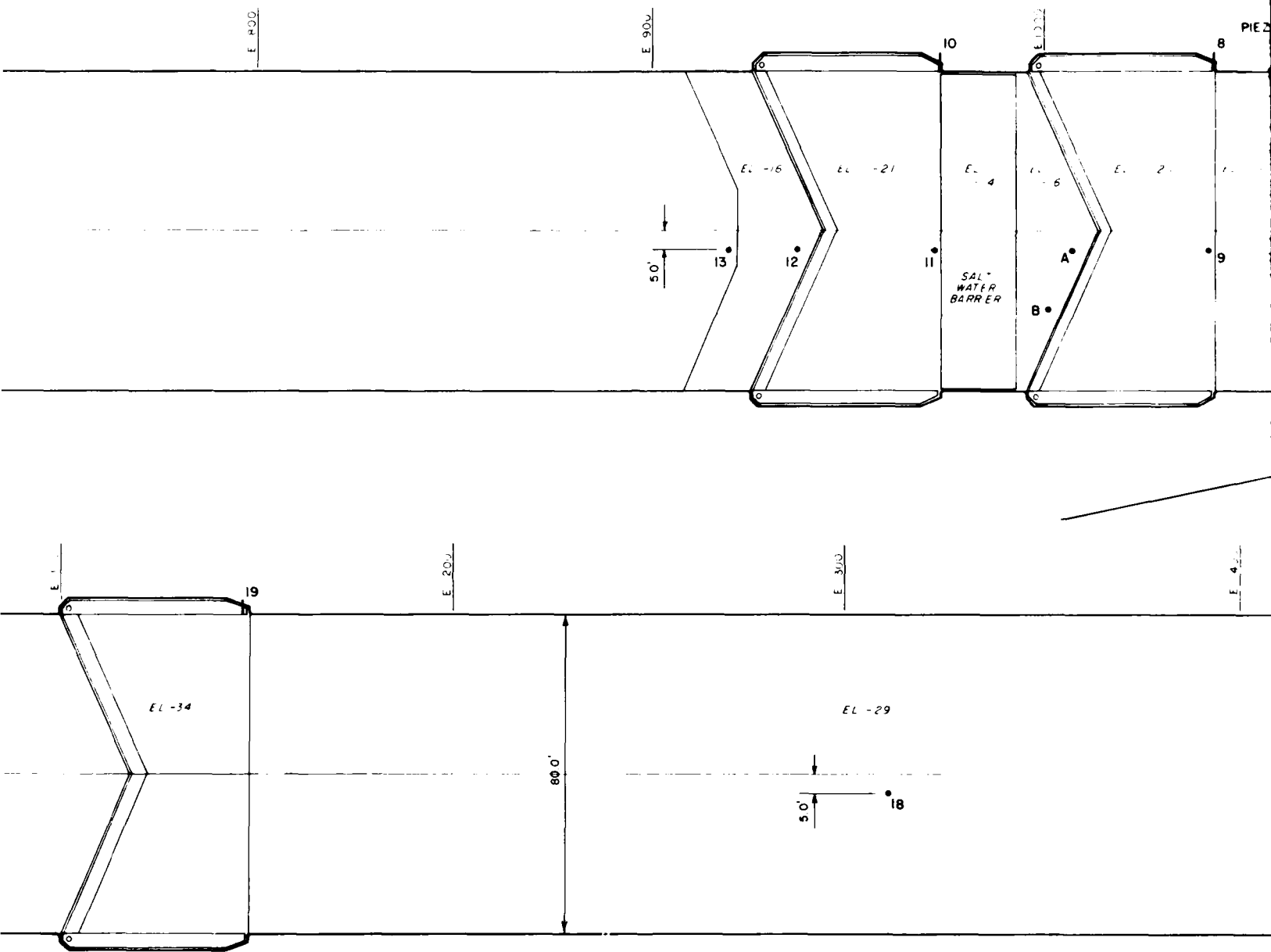
GENERAL LAYOUT

3

PLATE 2

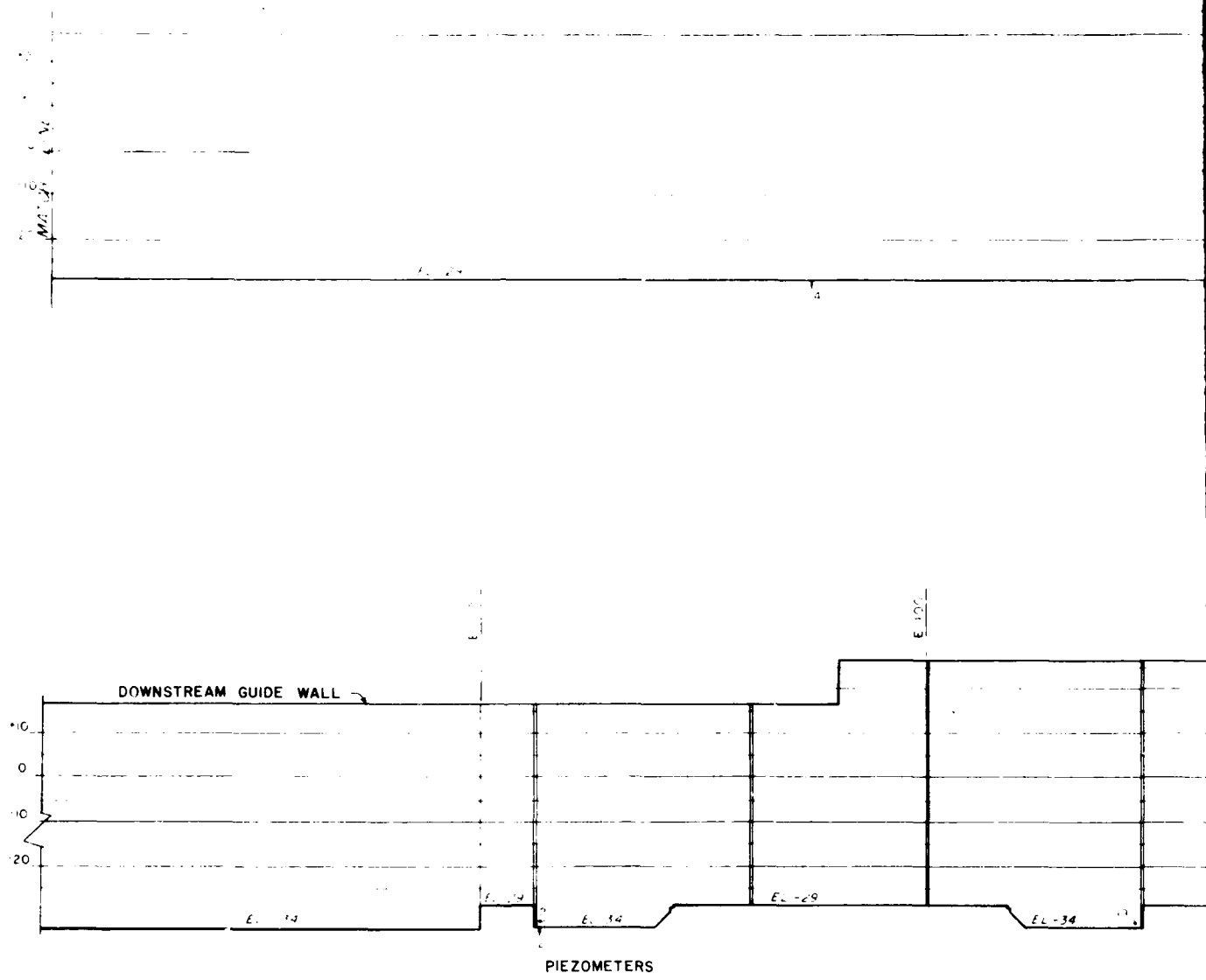


NOTE: PIEZOMETER LOCATIONS
LISTED IN TABLE A

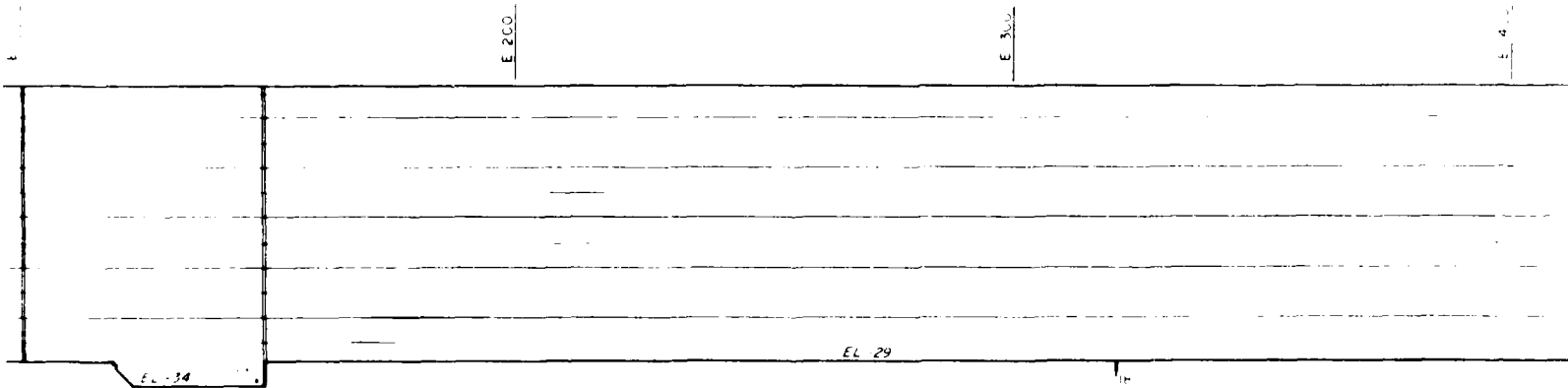
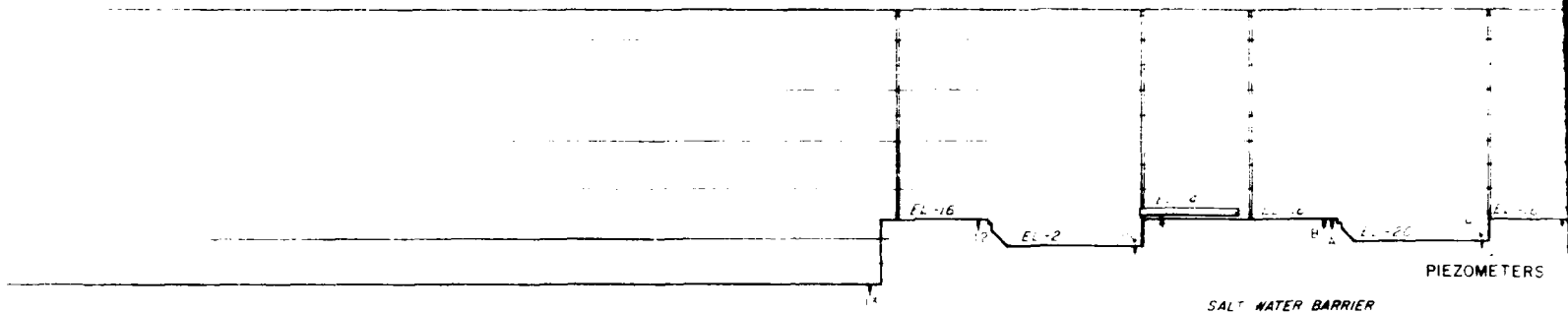


PLAN

2



NOTE: PIEZOMETER LOCATIONS
LISTED IN TABLE A

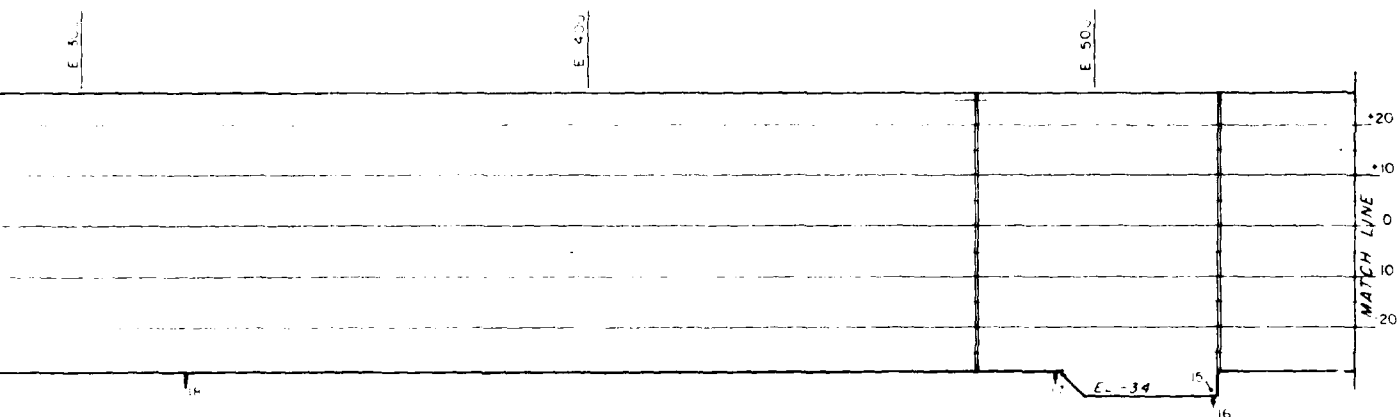
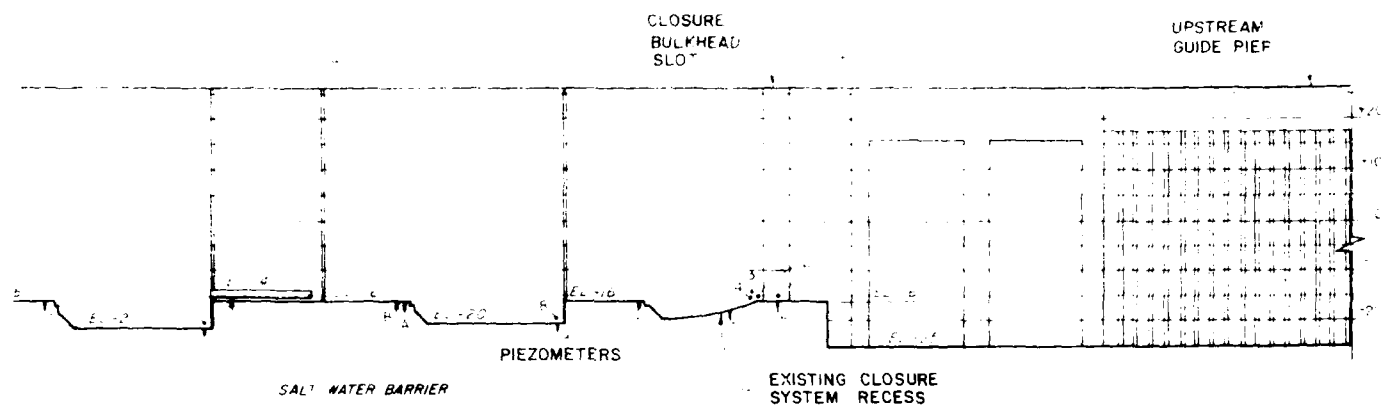


SECTION ALONG CENTER LINE OF LOCK

LEGEND

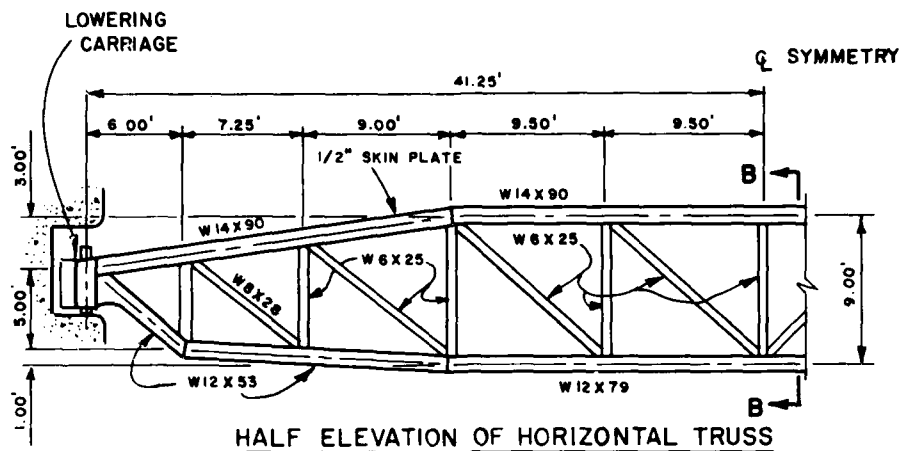
- WATER-SURFACE PROFILE ALONG RIGHT WALL
- (26) PRESSURE IN FEET OF WATER

2

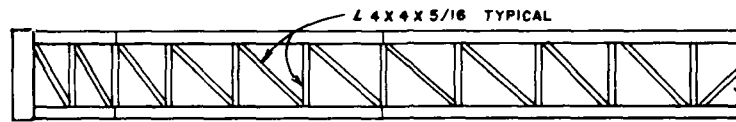


MODEL DETAILS AND
PIEZOMETER LOCATIONS

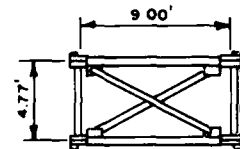
3



HALF ELEVATION OF HORIZONTAL TRUSS



HALF ELEVATION OF VERTICAL TRUSS

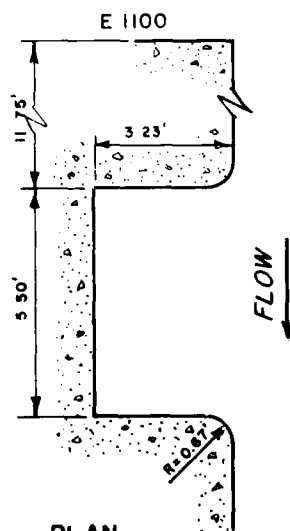


SECTION B-B

BOTTOM BULKHEAD

NOTE

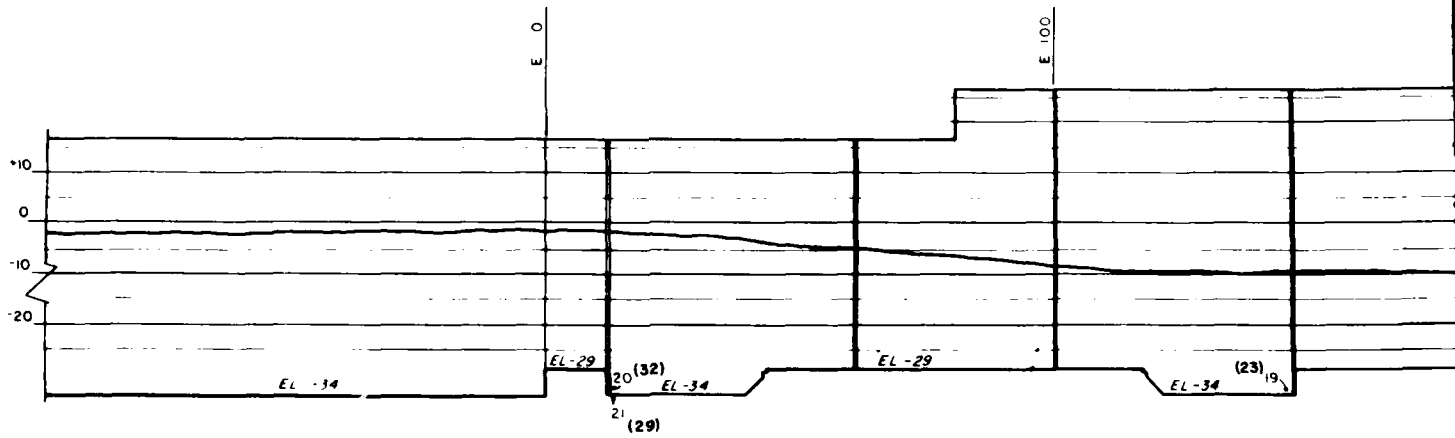
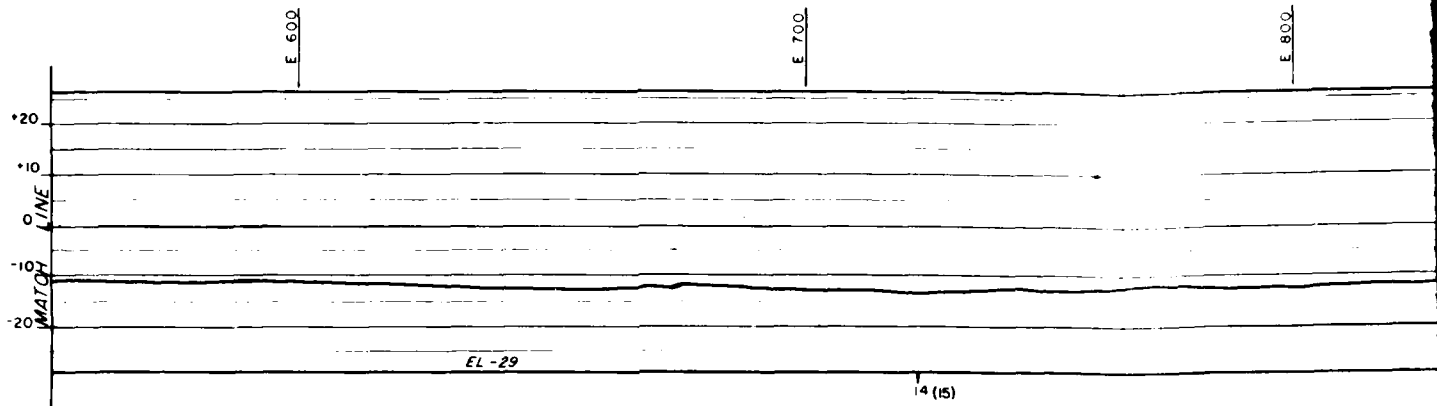
TOP FOUR BULKHEADS HAVE LIGHTER MEMBERS,
OTHER TWO SAME AS BOTTOM.

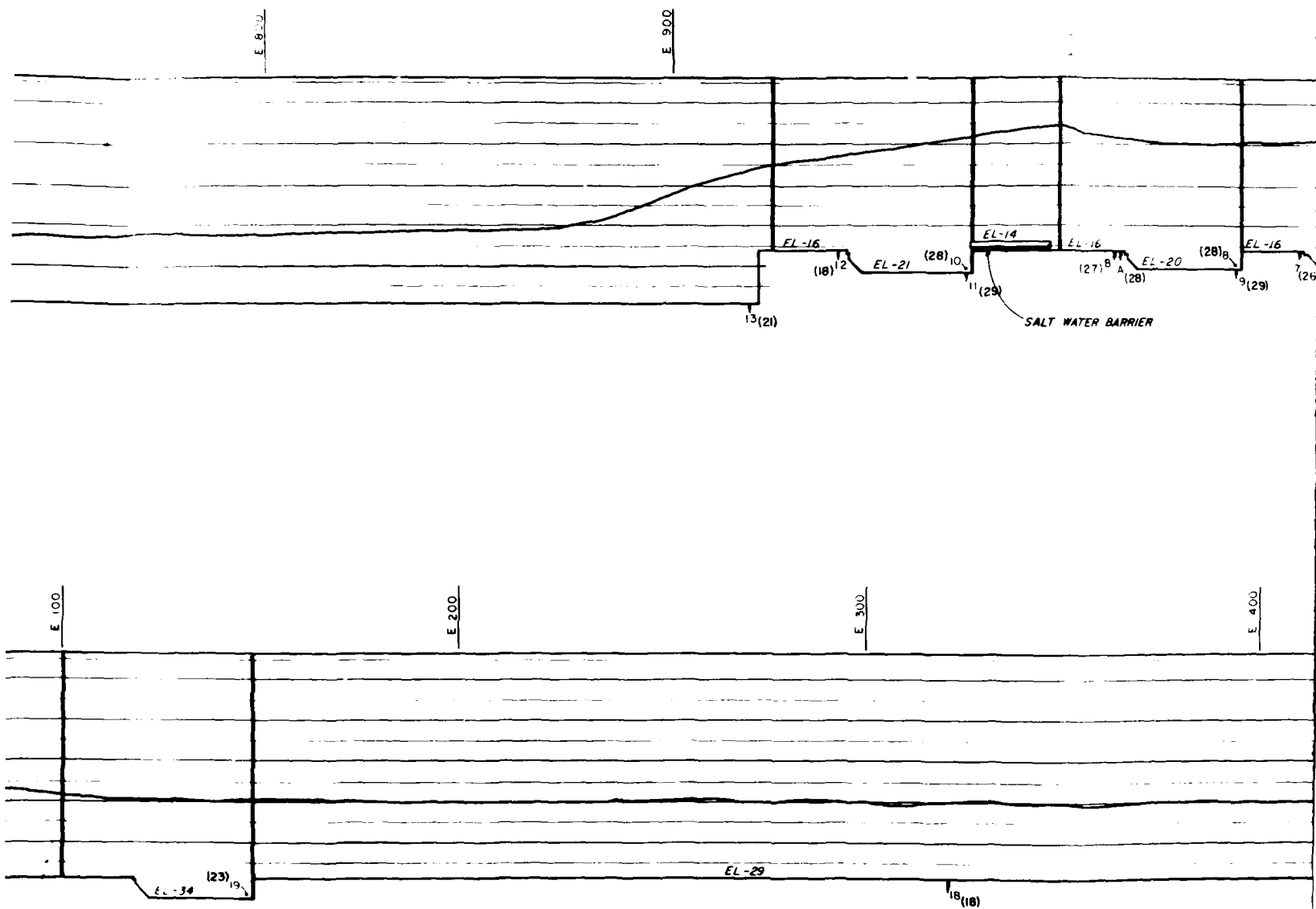


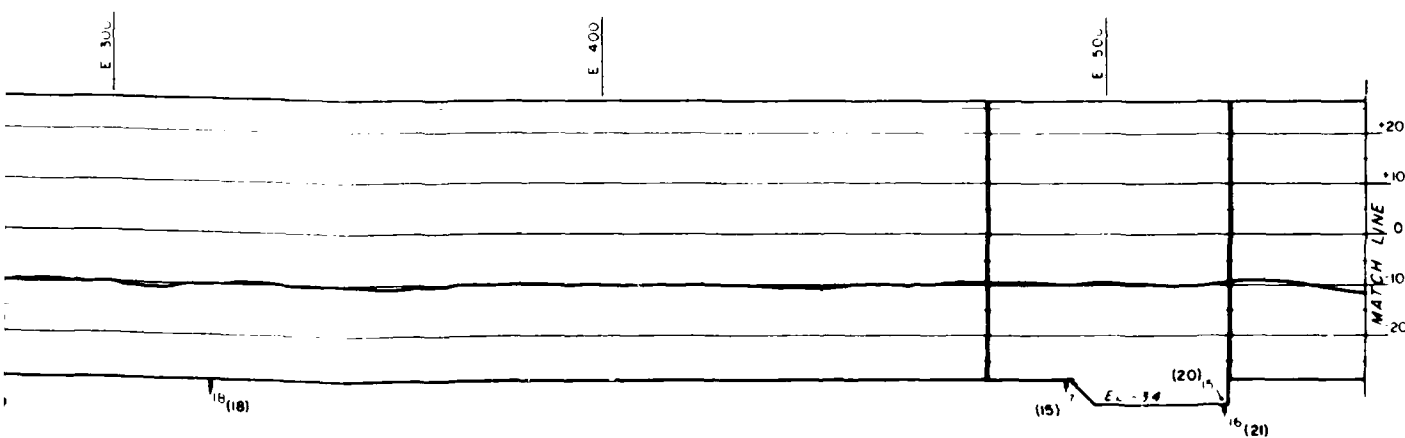
PLAN

BULKHEAD SLOT

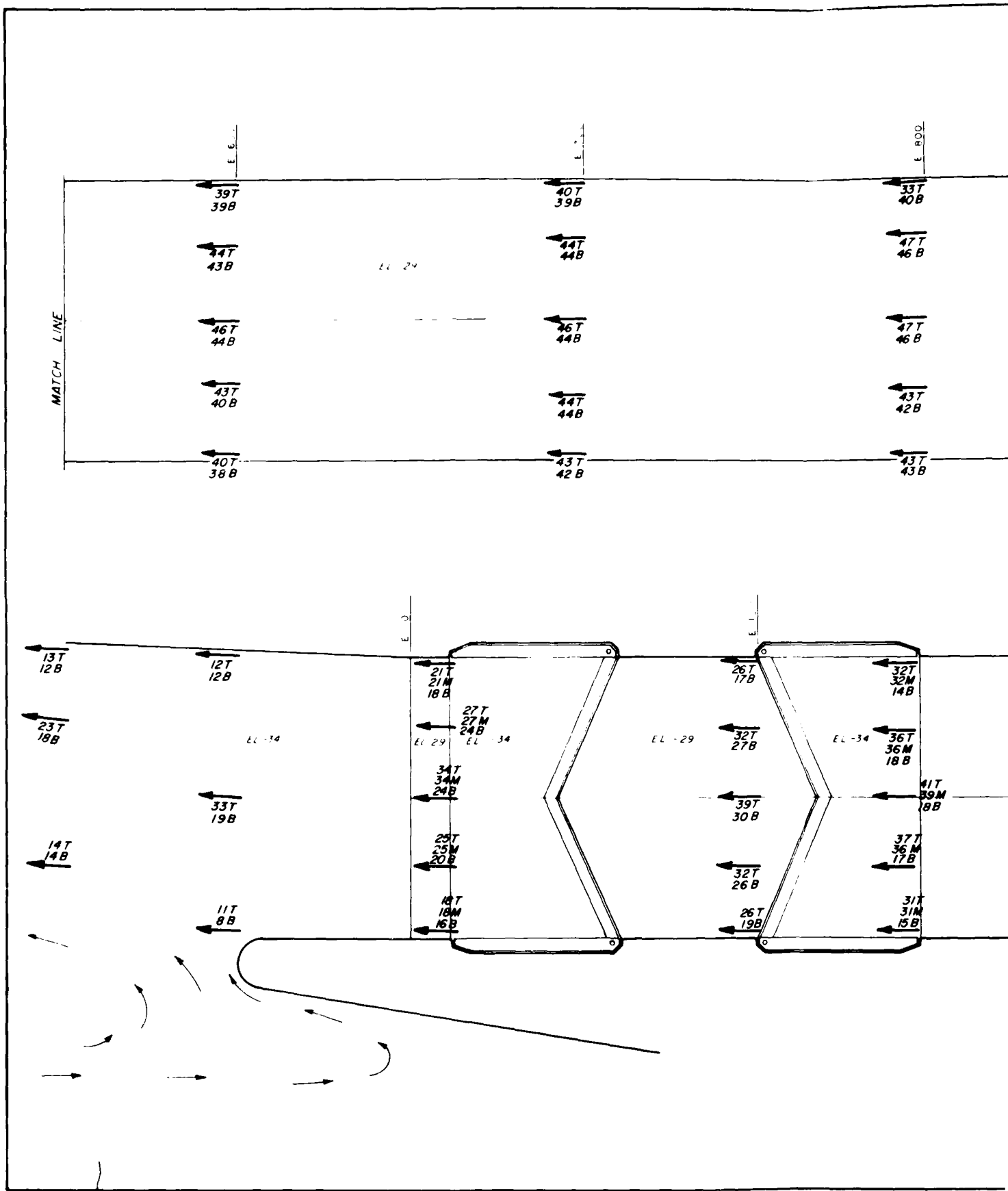
BOTTOM BULKHEAD
AND SLOT

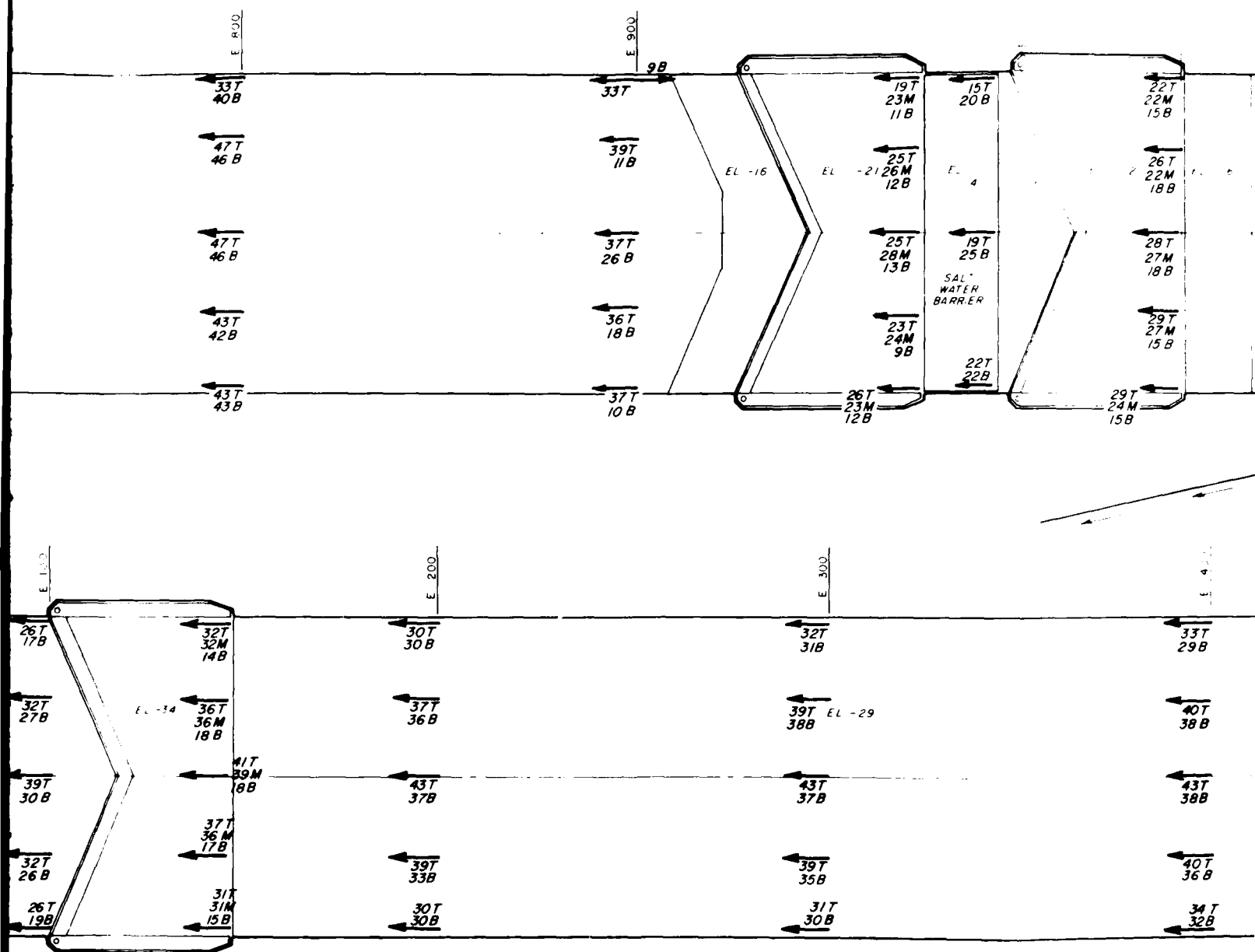


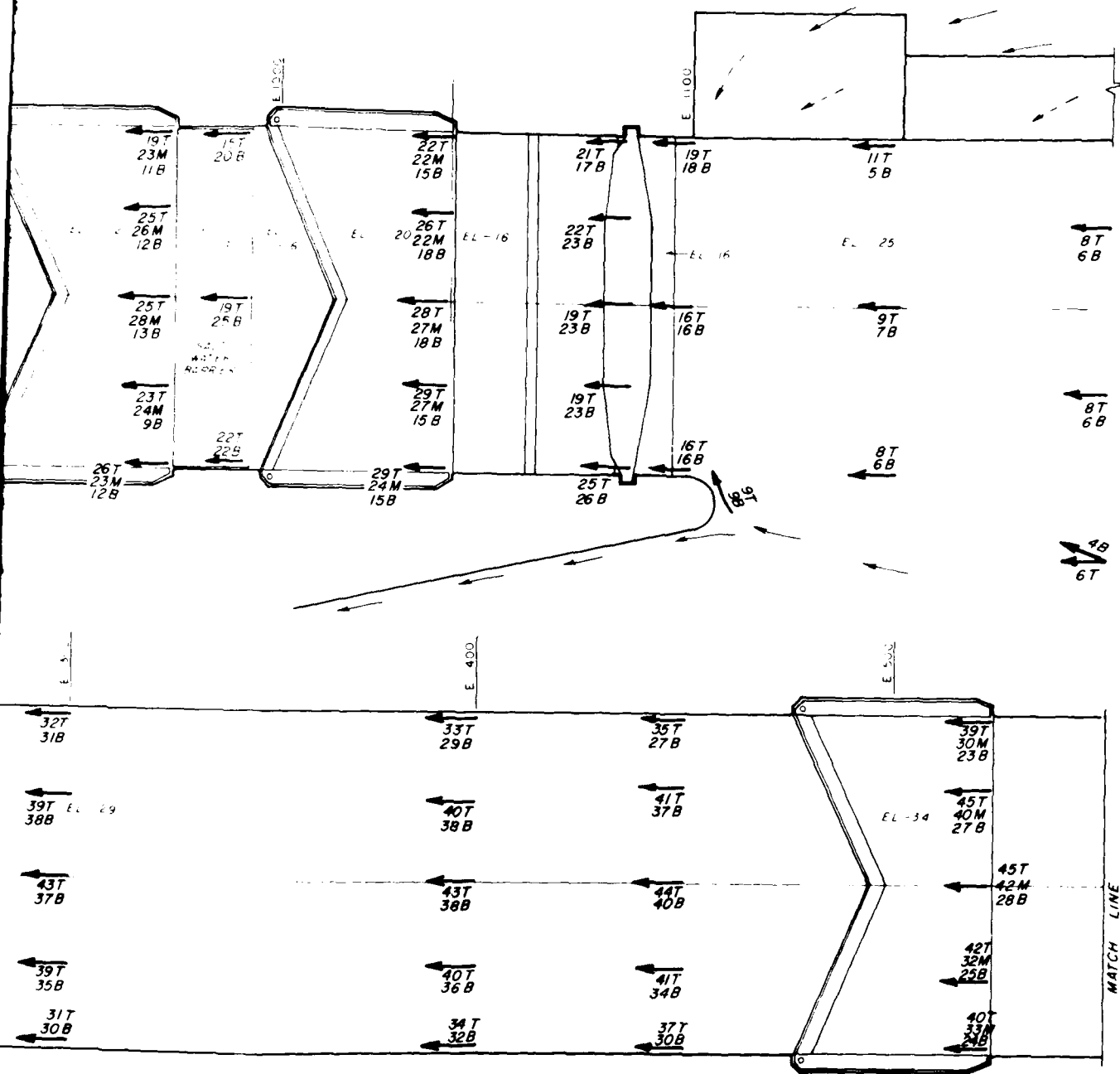




FREE FLOW





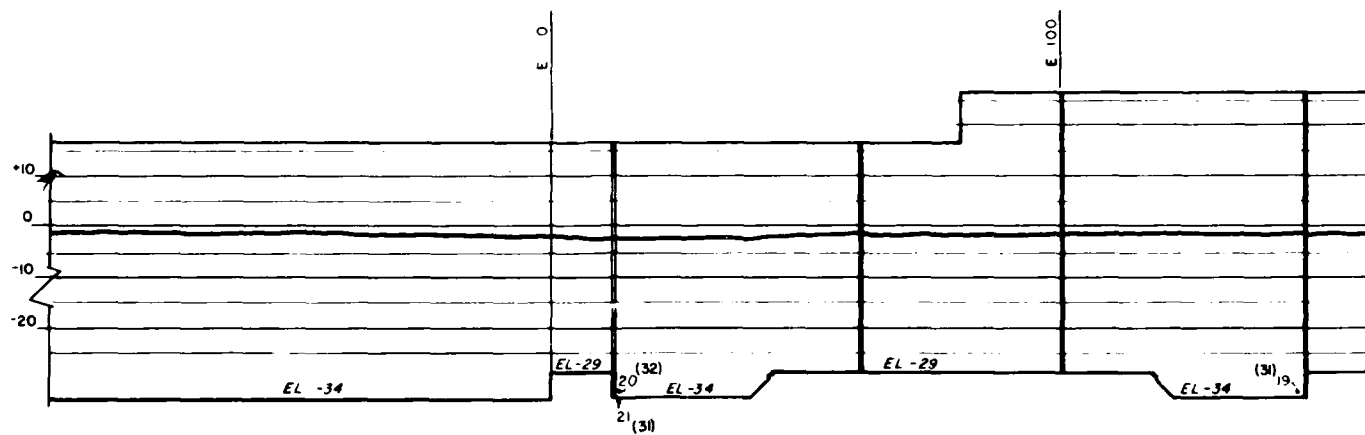
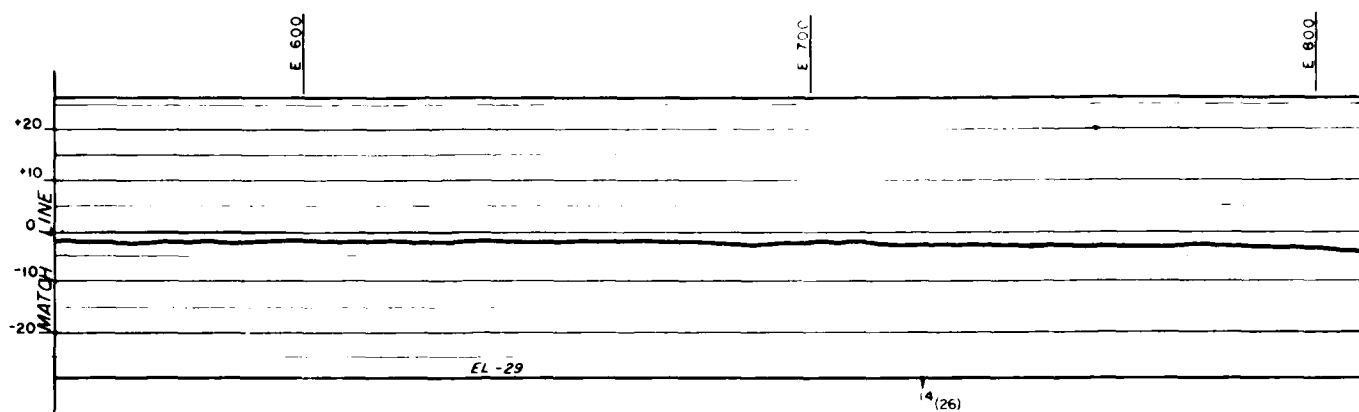


LOCK DISCHARGE 48,700 CFS

VELOCITIES

FOREBAY EL 22; TAILWATER EL -2
FREE FLOW

3



E 800

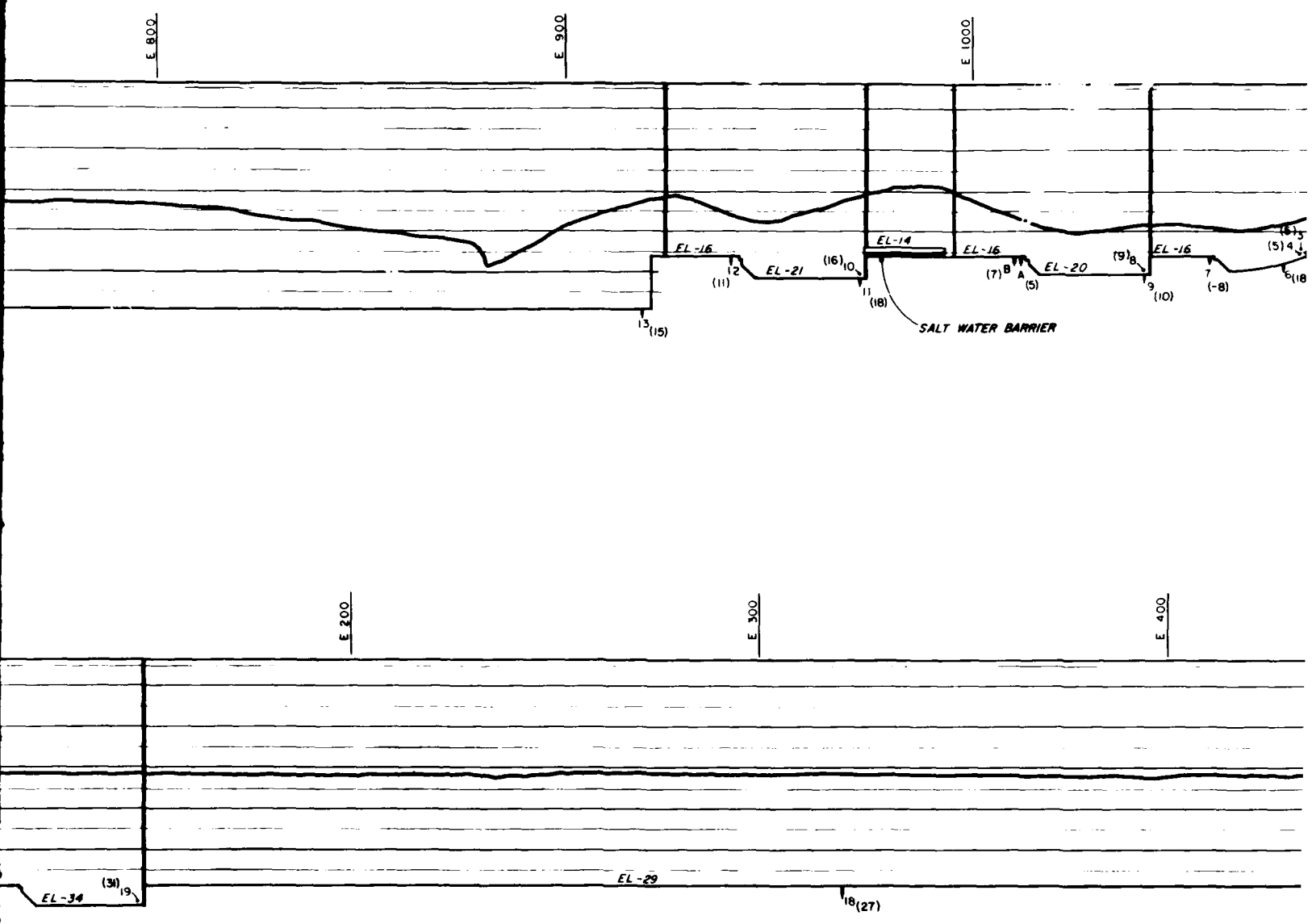
E 900

E 1000

E 200

E 300

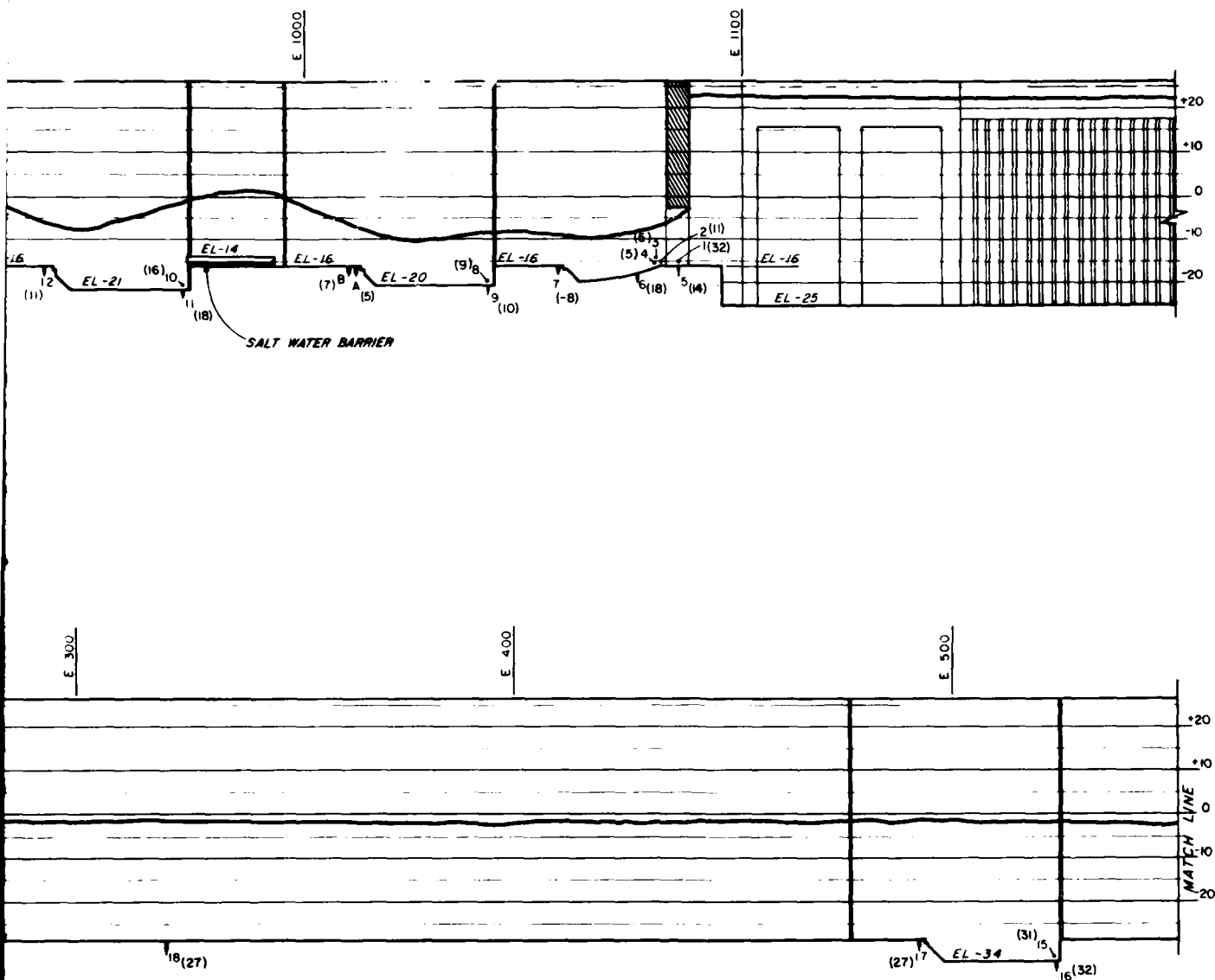
E 400



LEGEND

- WATER-SURFACE PROFILE ALONG RIGHT WALL
- (26) PRESSURE IN FEET OF WATER

2

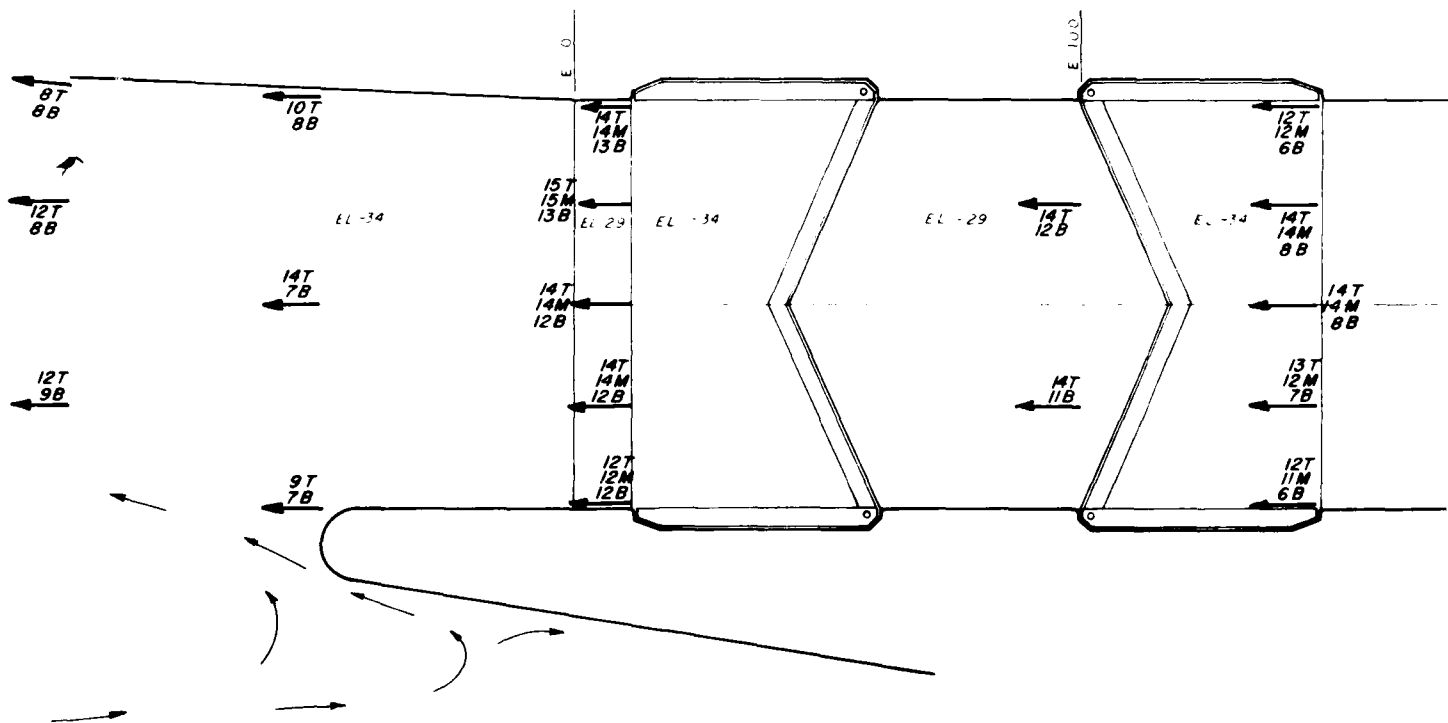
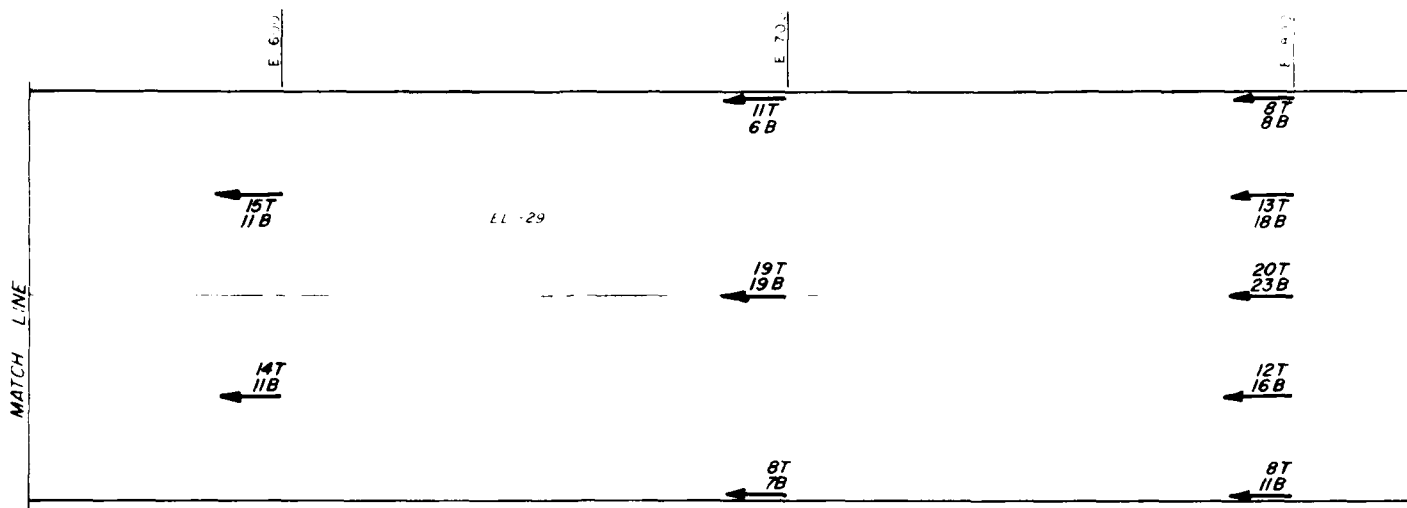


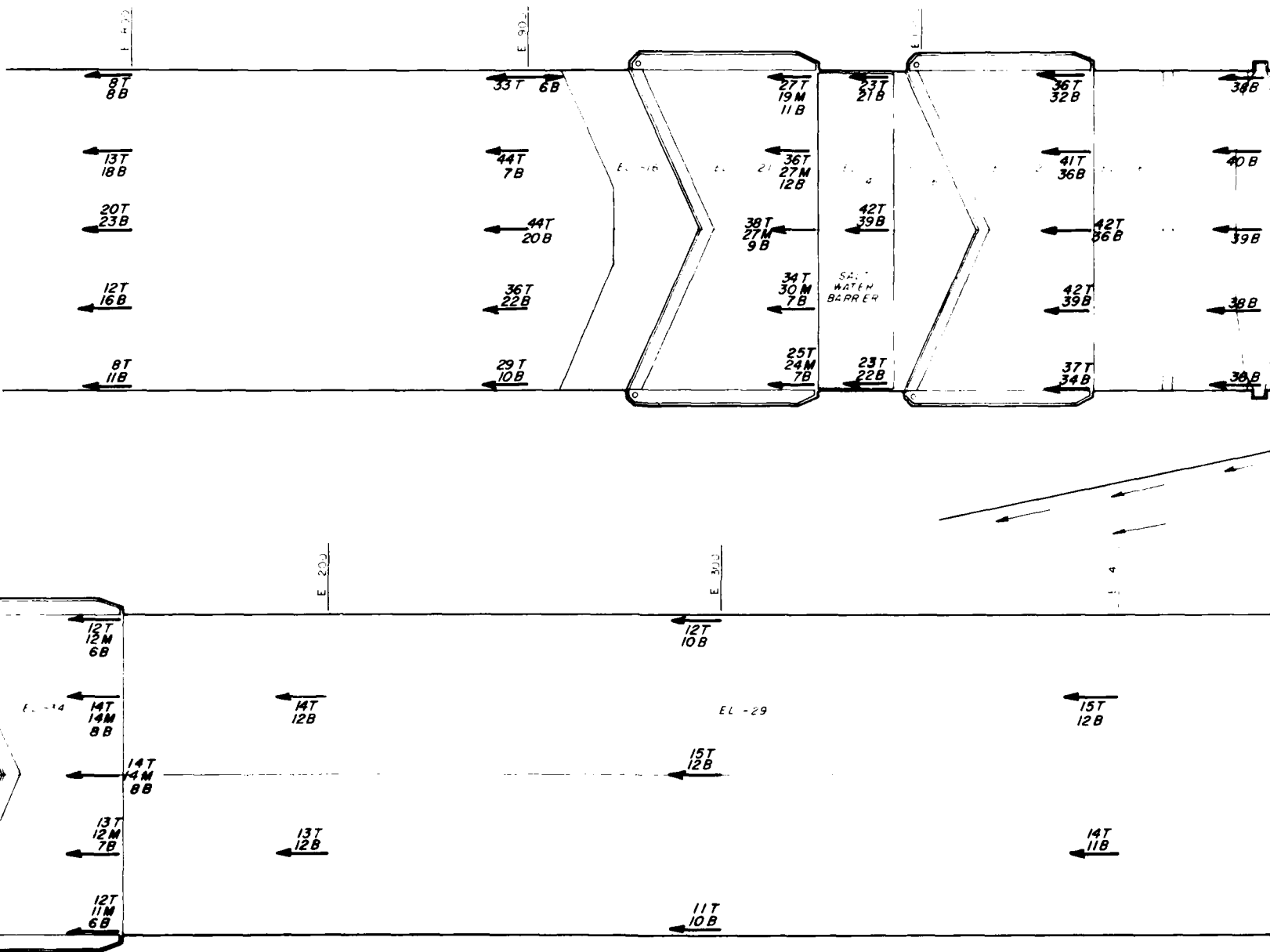
LOCK DISCHARGE 27 860 CFS

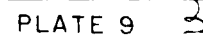
WATER SURFACE AND PRESSURES

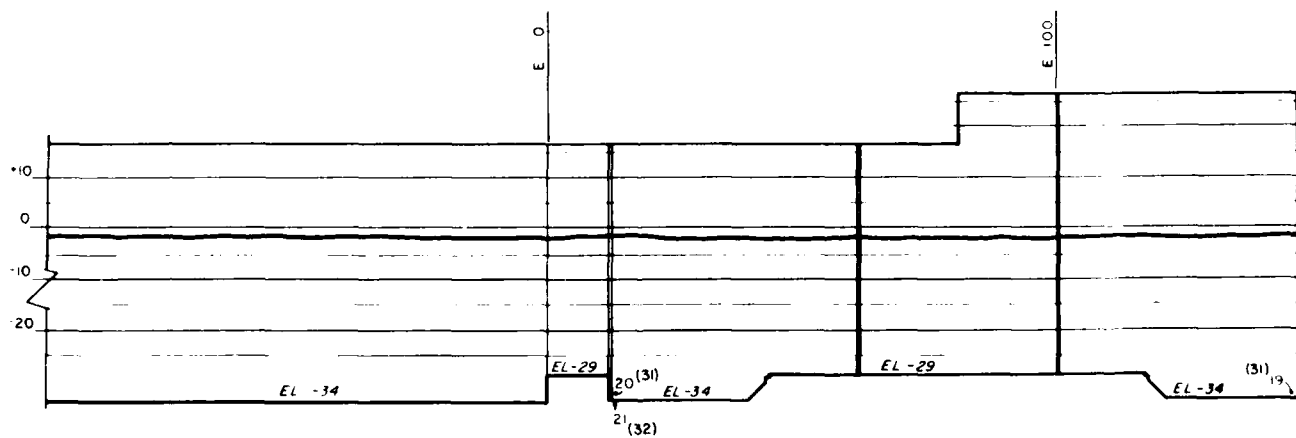
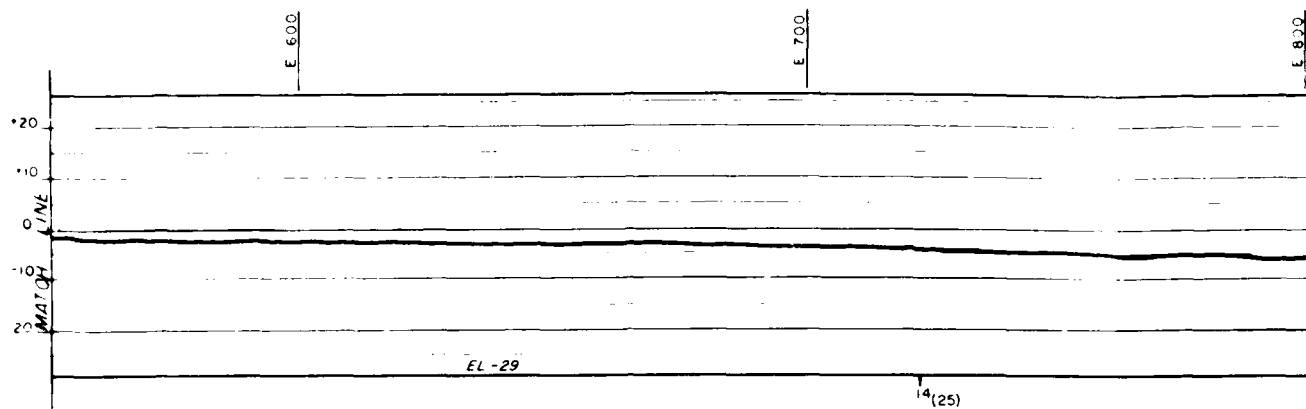
FOREBAY EL 22; TAILWATER EL -2

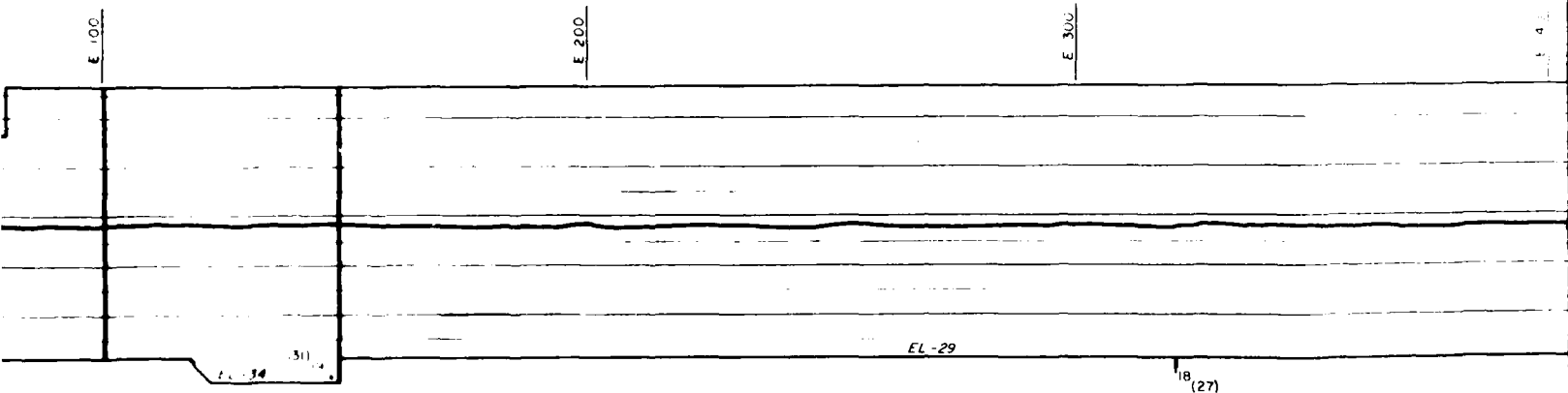
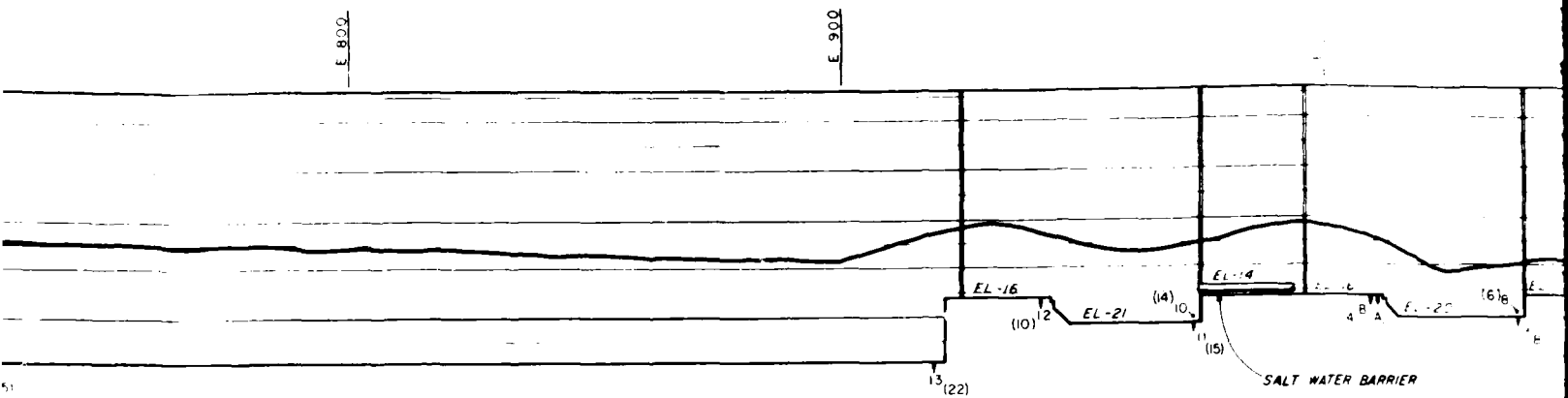
BULKHEAD OPEN 13 FT





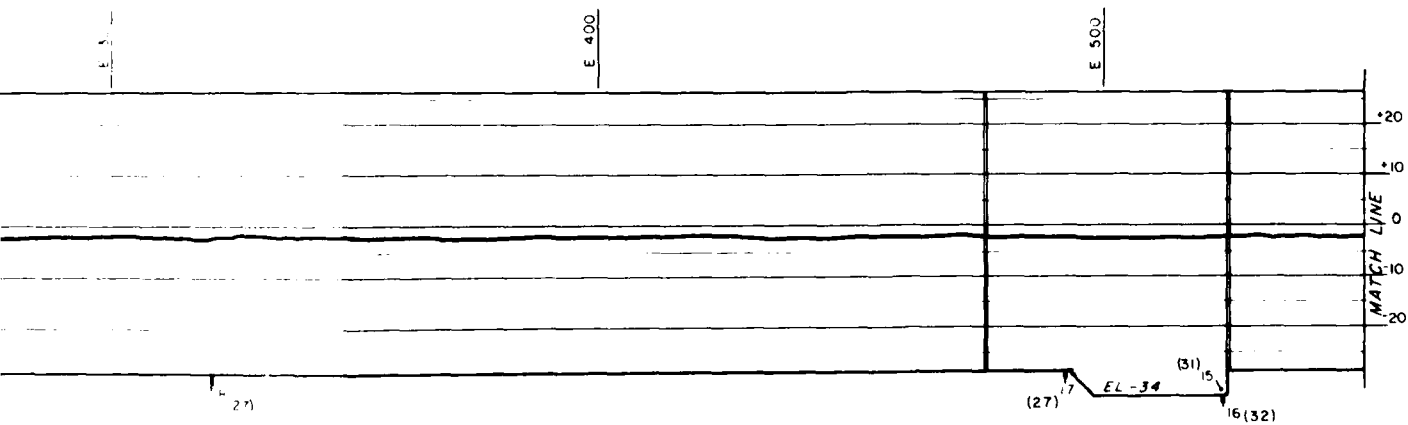
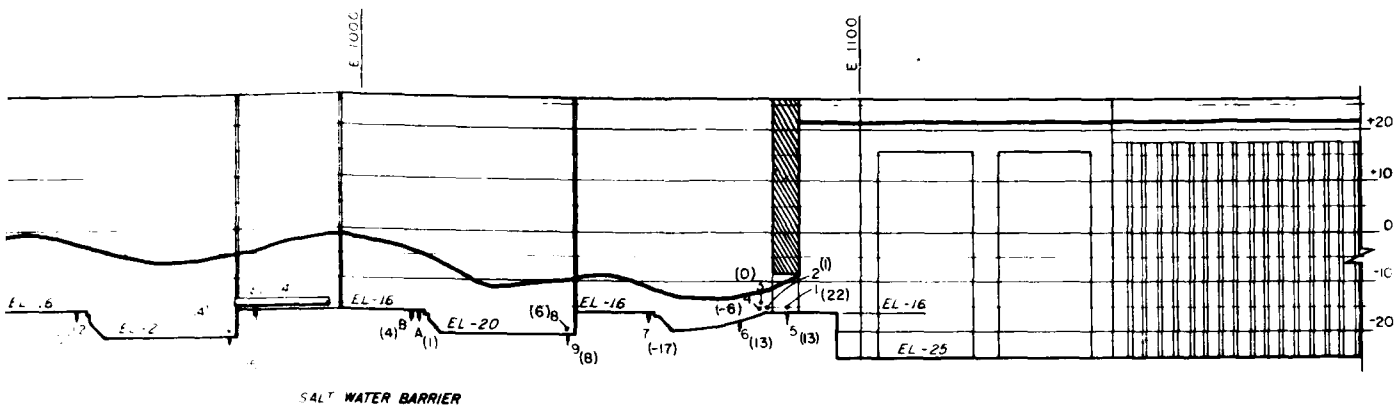






LEGEND

- WATER-SURFACE PROFILE ALONG RIGHT WALL
- (26) PRESSURE IN FEET OF WATER

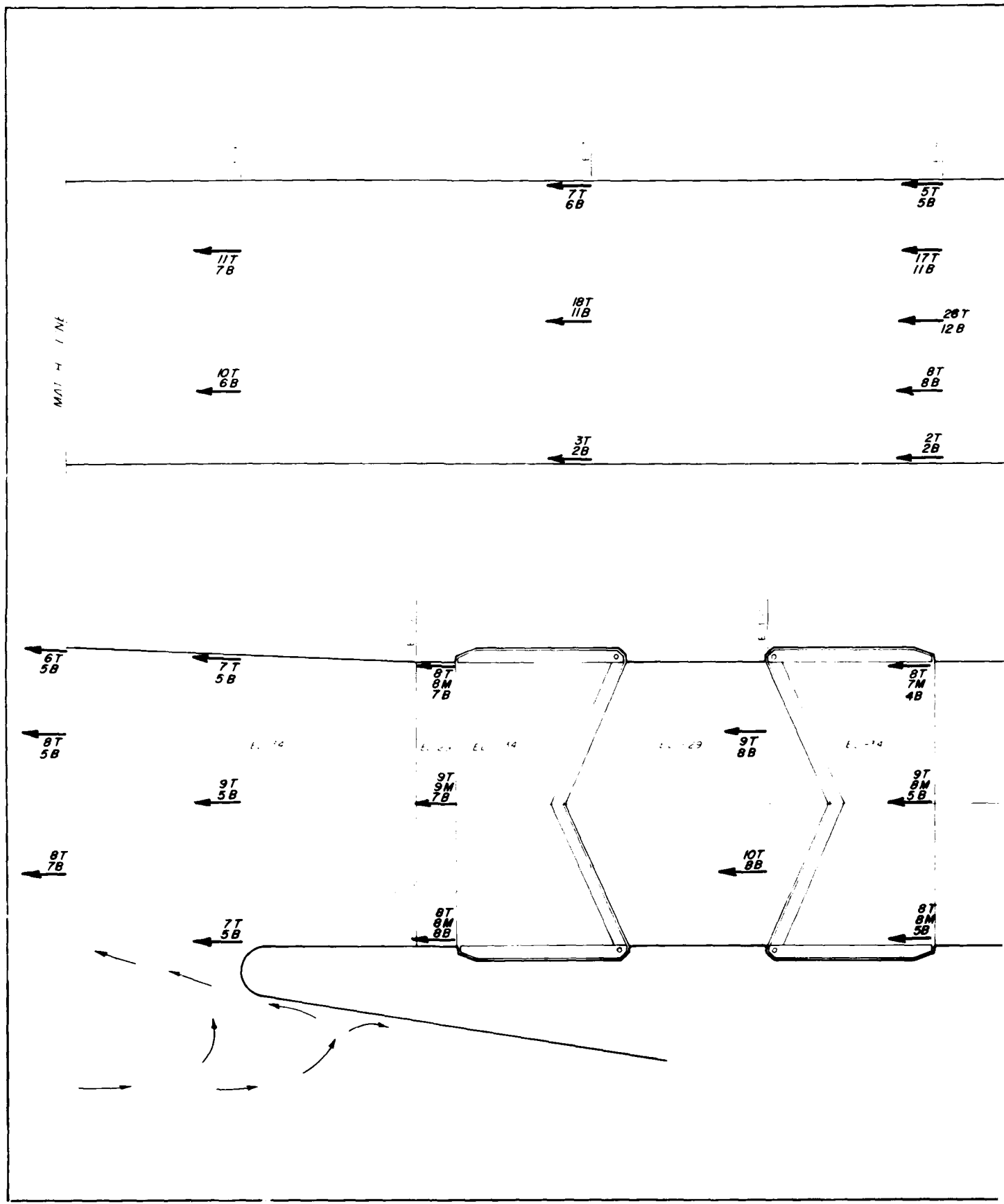


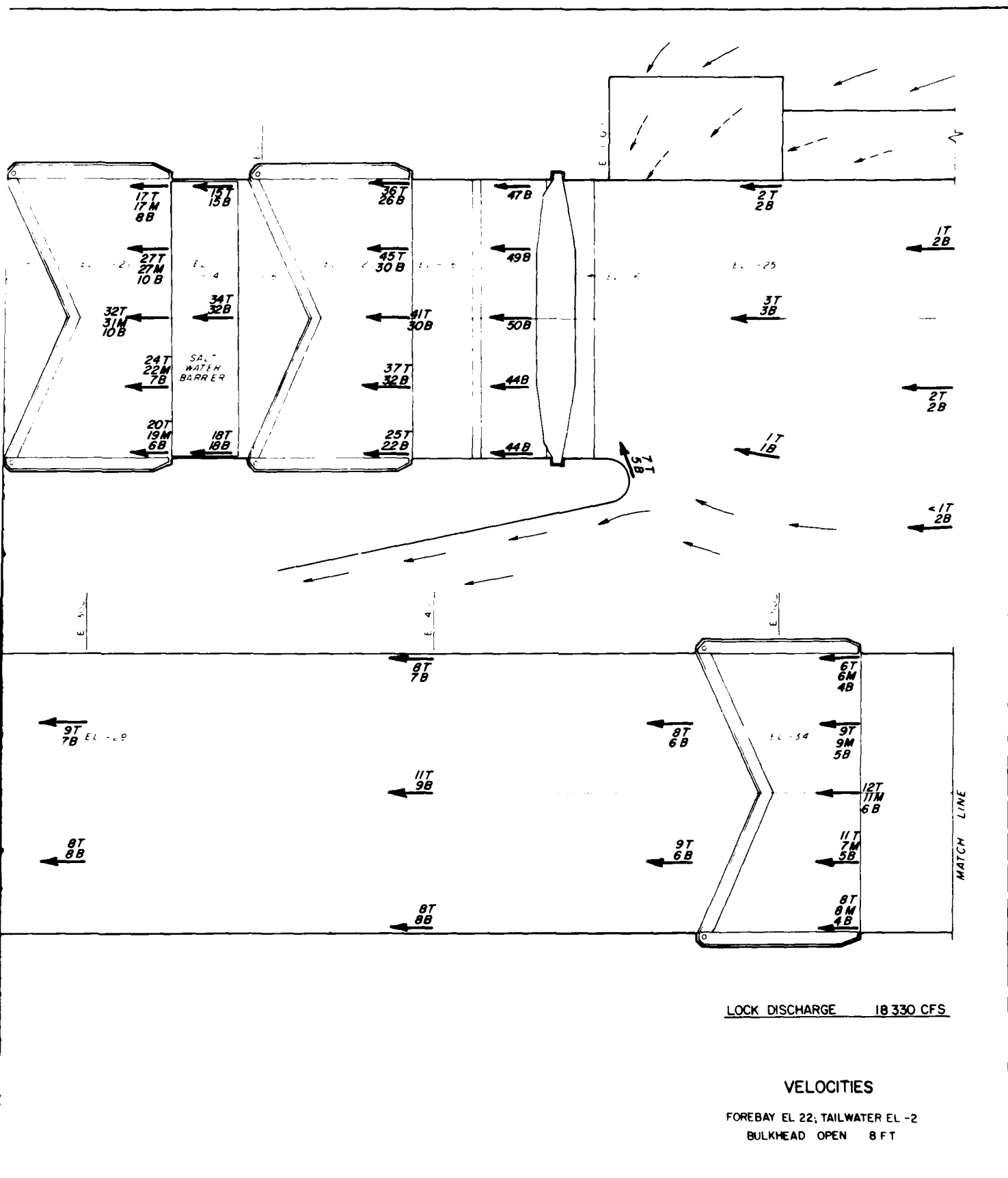
LOCK DISCHARGE 18 330 CFS

WATER SURFACE AND PRESSURES

FOREBAY EL 22; TAILWATER EL -2

BULKHEAD OPEN 8 FT

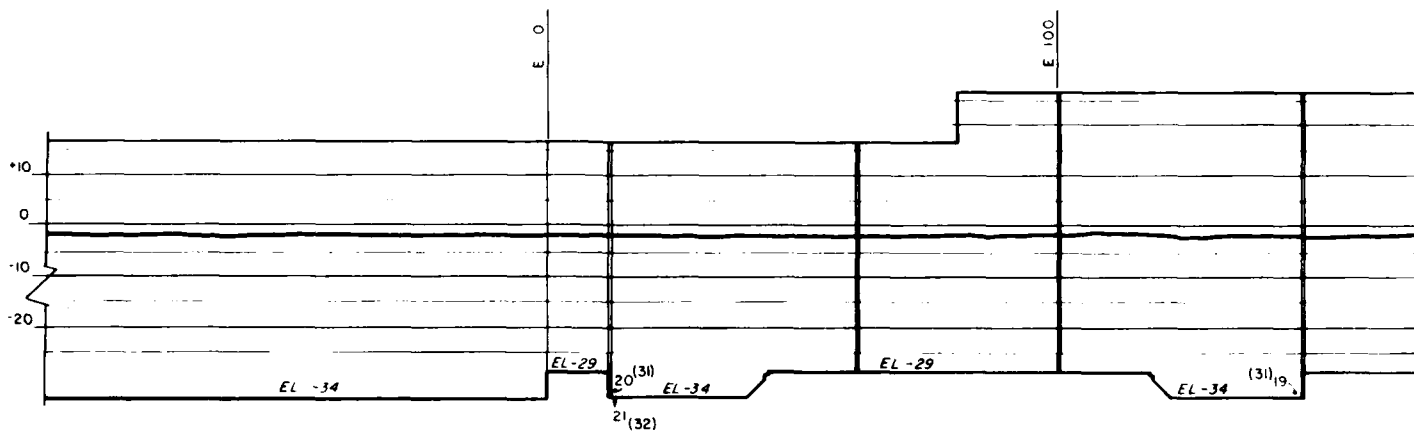
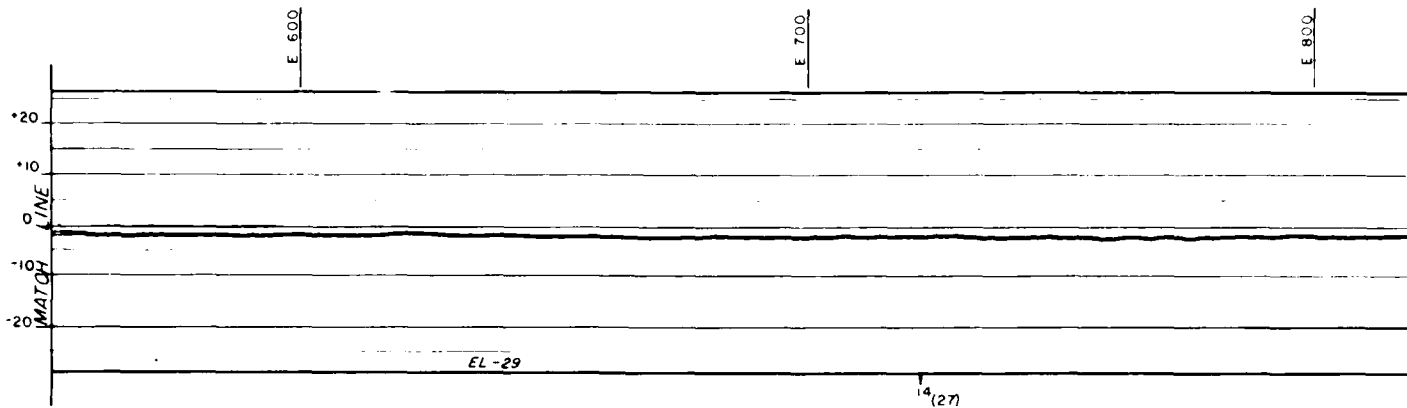


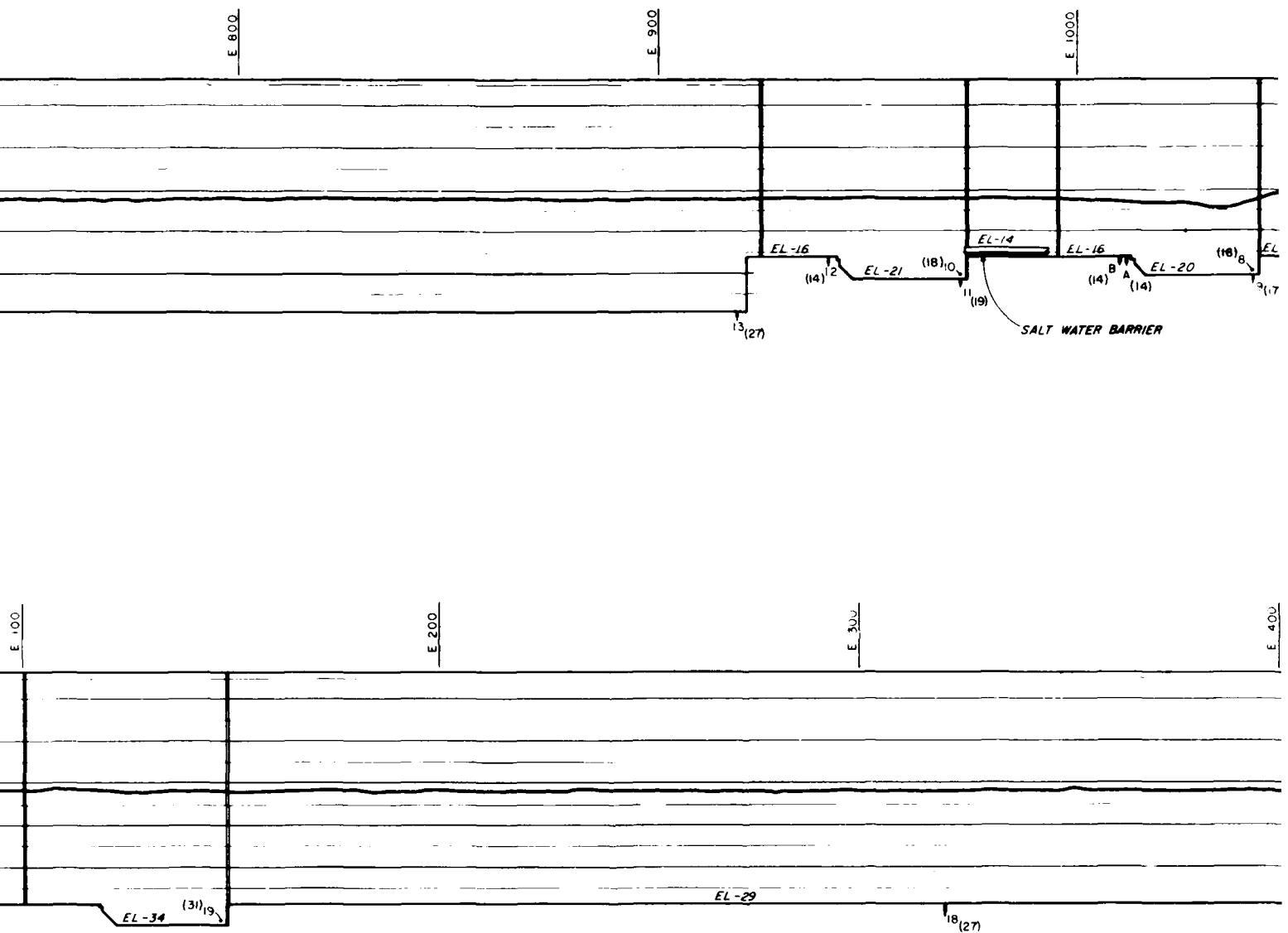


LOCK DISCHARGE 18 330 CFS

VELOCITIES

FOREBAY EL 22; TAILWATER EL -2
BULKHEAD OPEN 8 FT

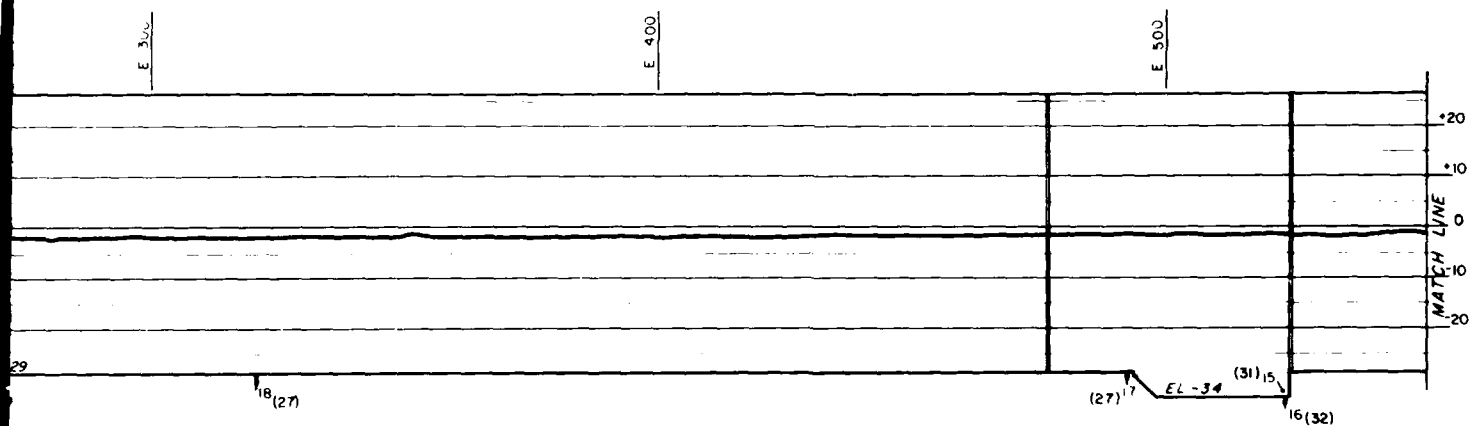
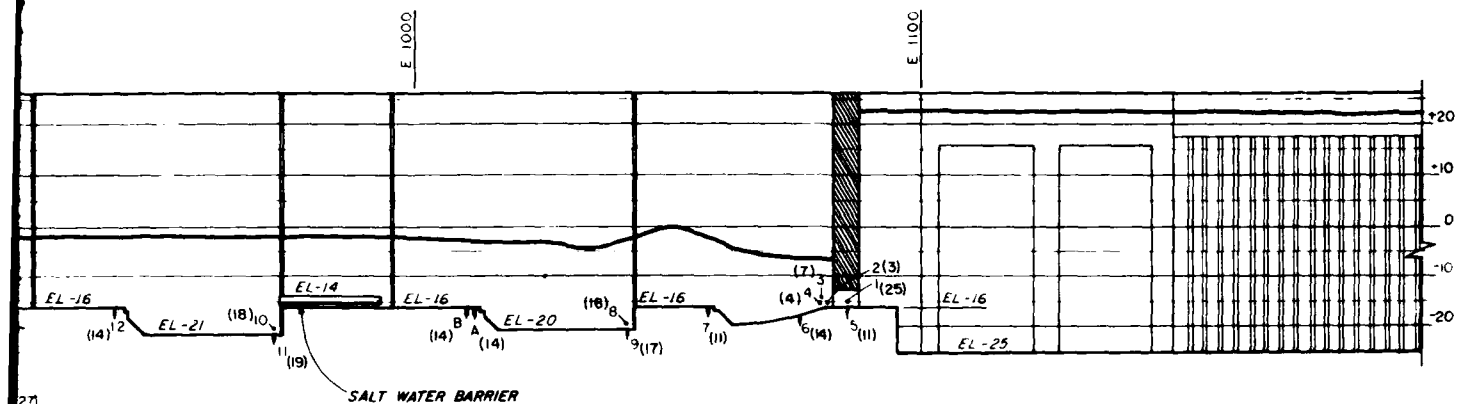




LEGEND

- WATER-SURFACE PROFILE ALONG RIGHT WALL
- (26) PRESSURE IN FEET OF WATER

2



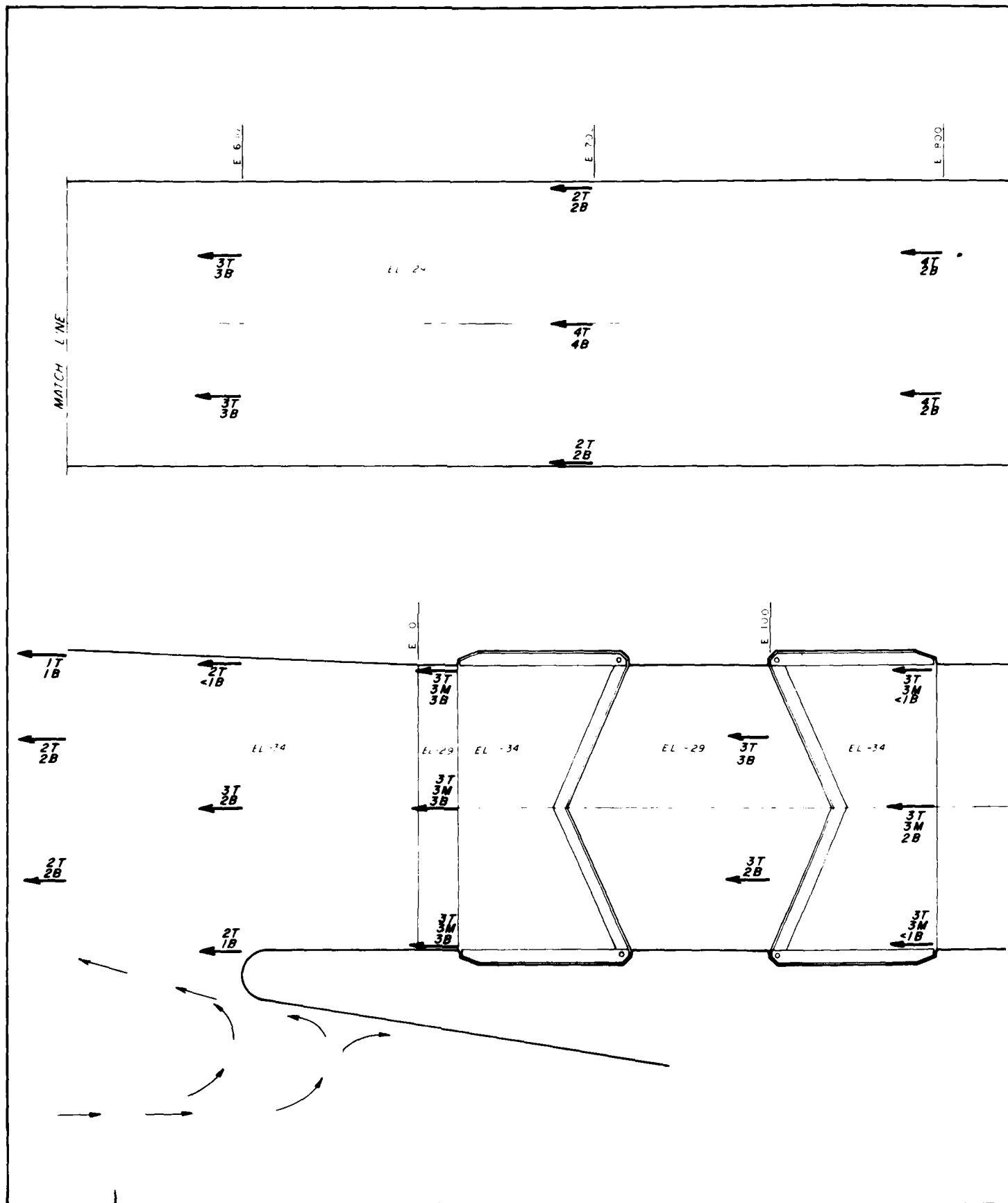
LOCK DISCHARGE 6 430 CFS

WATER SURFACE AND PRESSURES

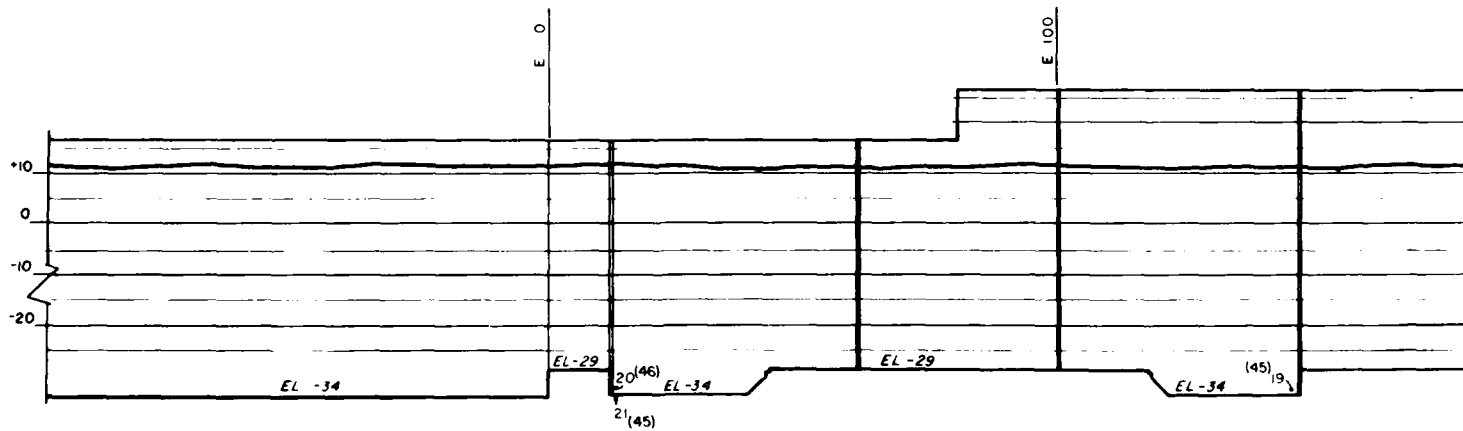
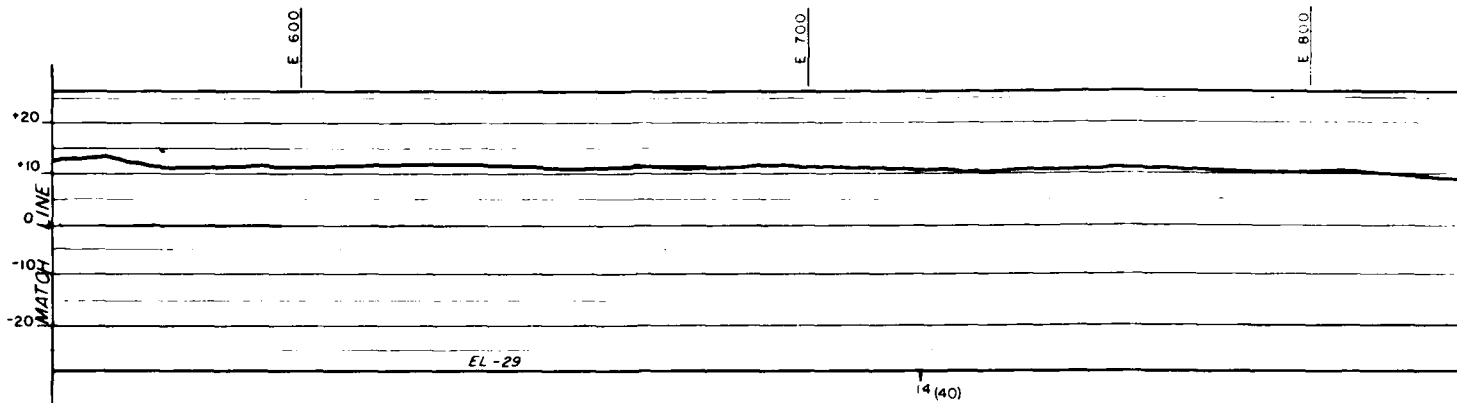
FOREBAY EL 22; TAILWATER EL -2

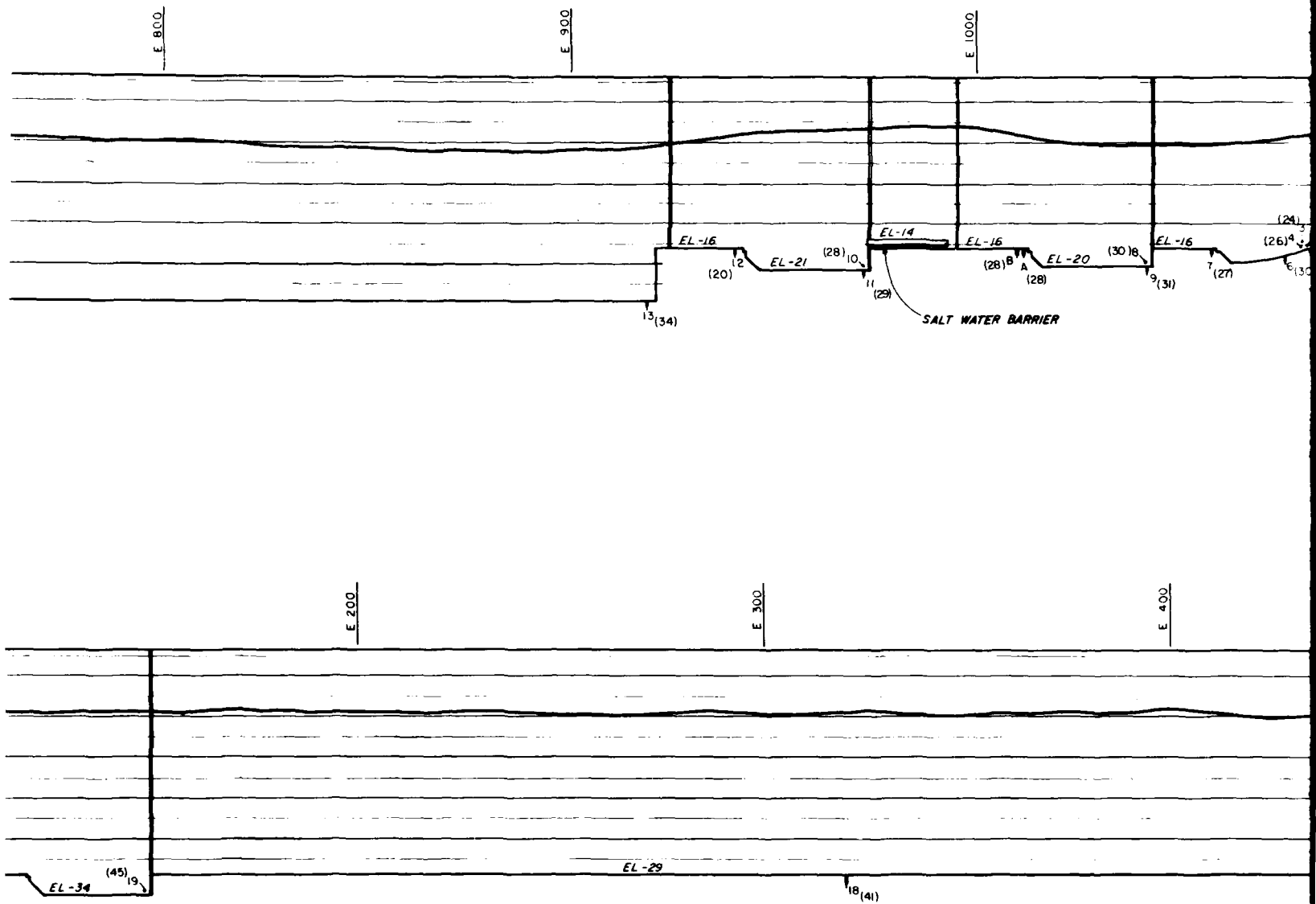
BULKHEAD OPEN 3 FT

3





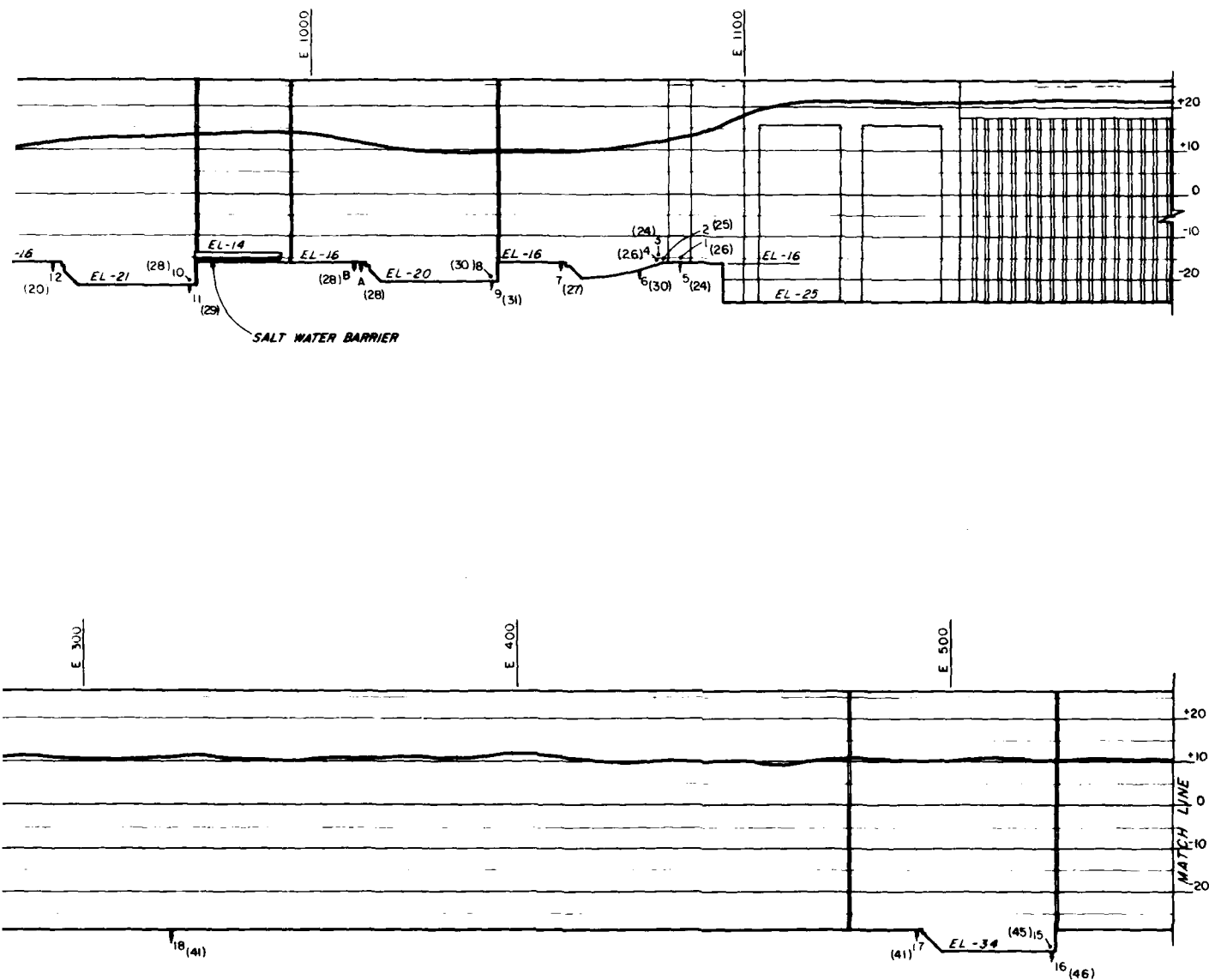




LEGEND

- WATER-SURFACE PROFILE ALONG RIGHT WALL
- (26) PRESSURE IN FEET OF WATER

2



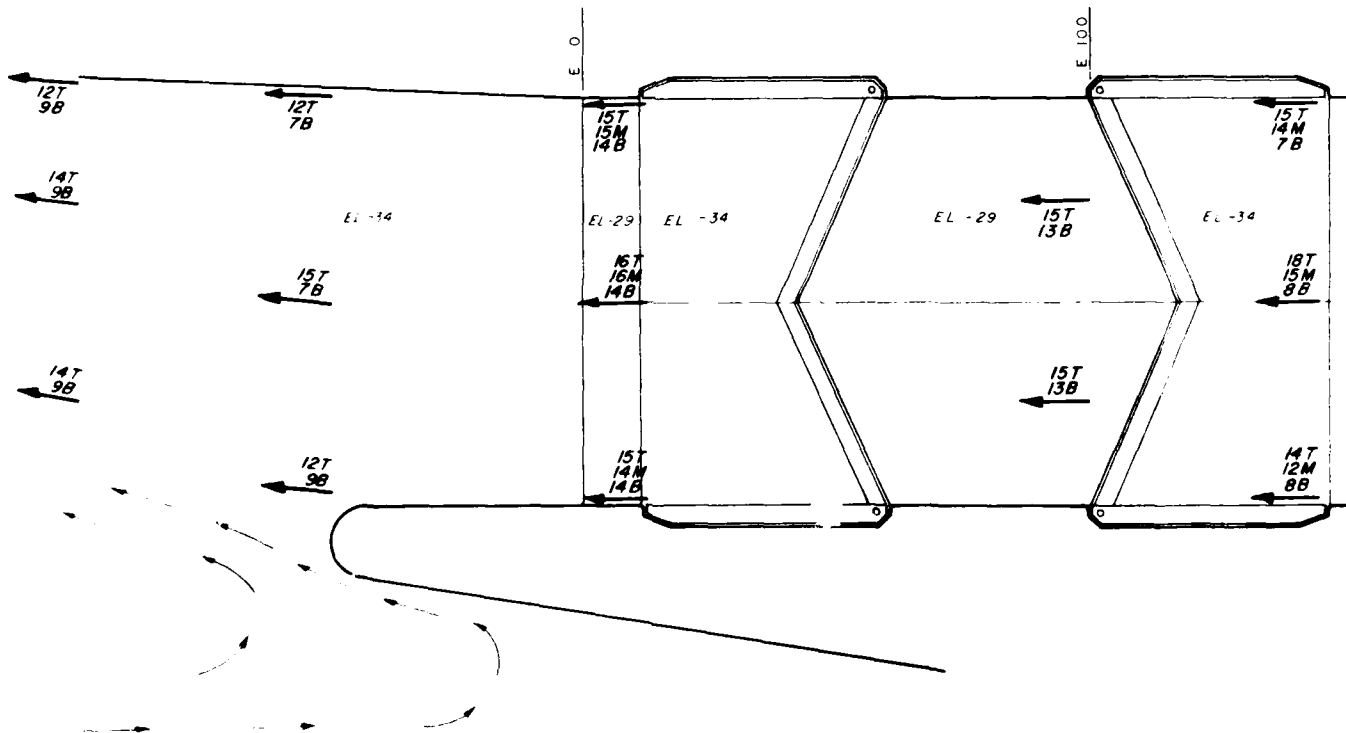
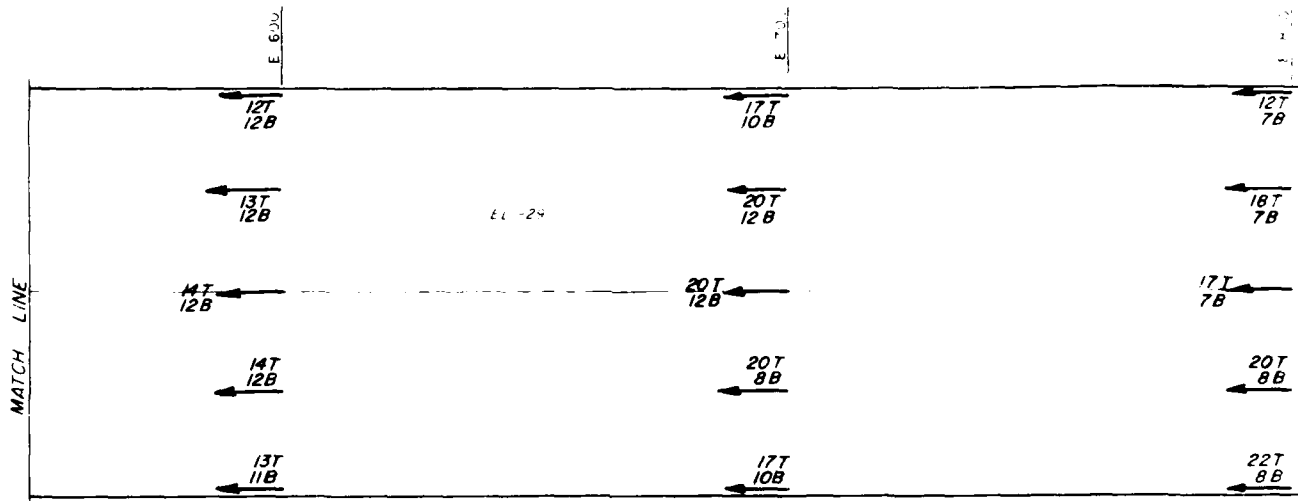
LOCK DISCHARGE 48 700 CFS

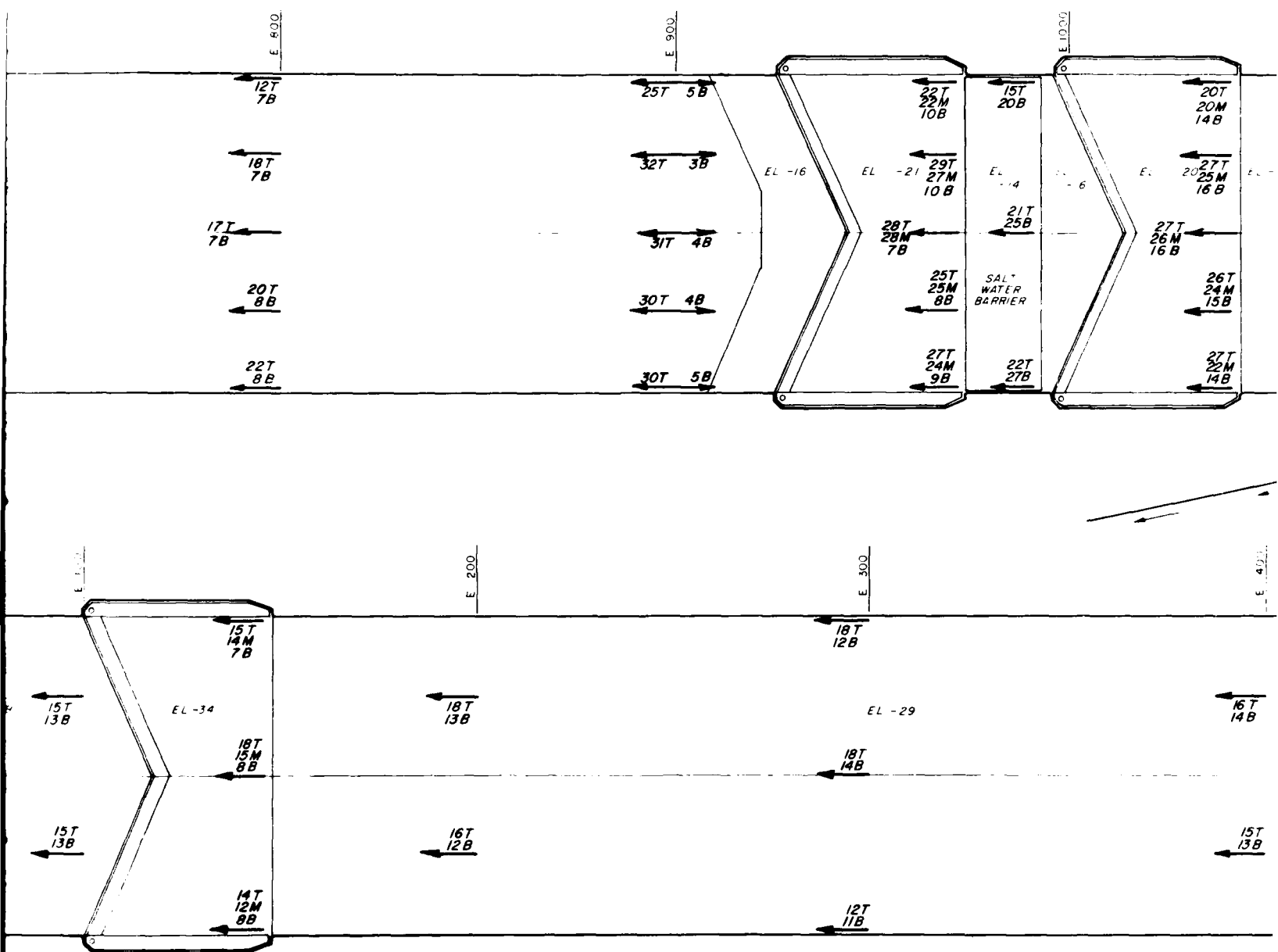
WATER SURFACE AND PRESSURES

FOREBAY EL 22, TAILWATER EL +12

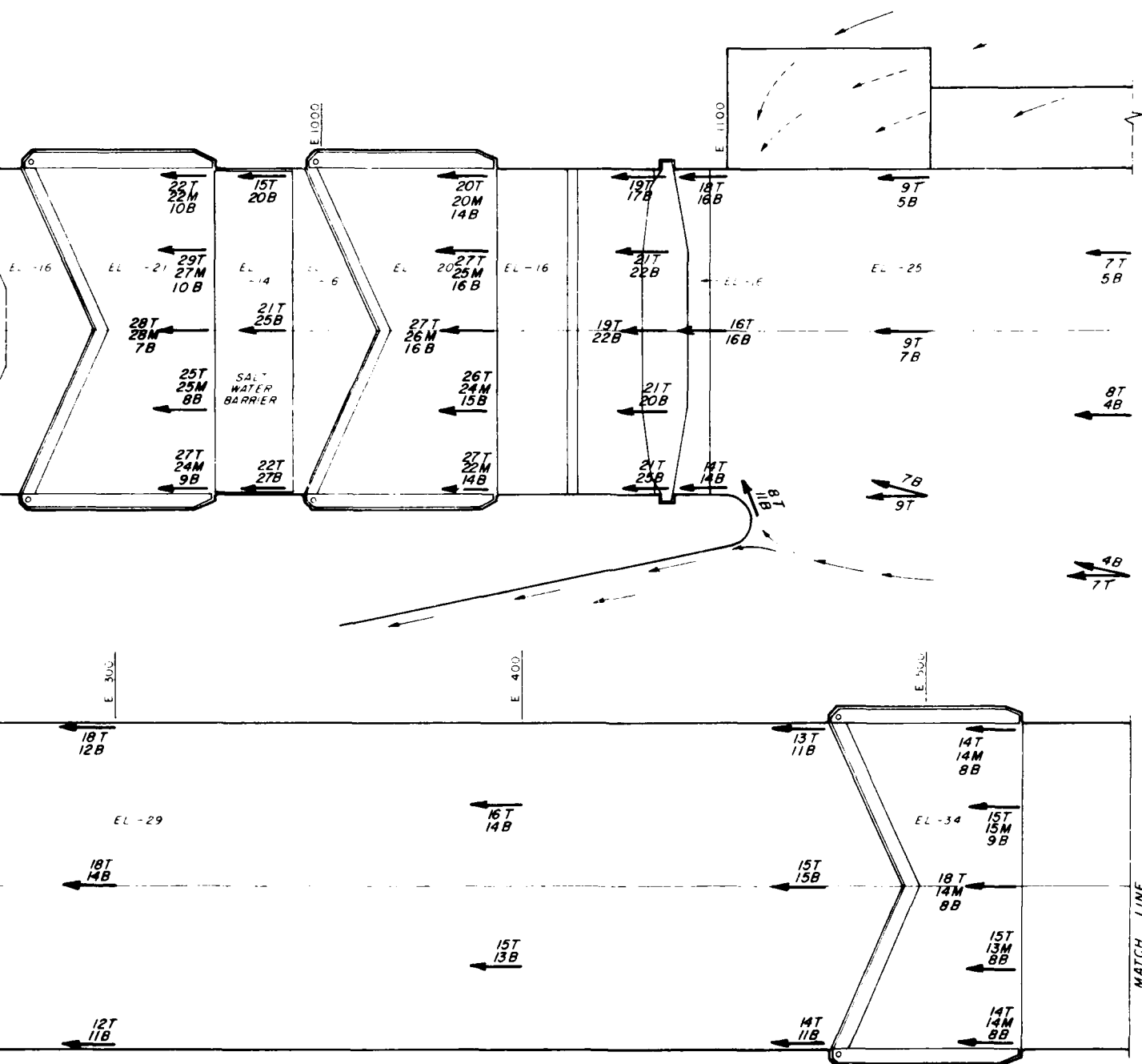
FREE FLOW

3





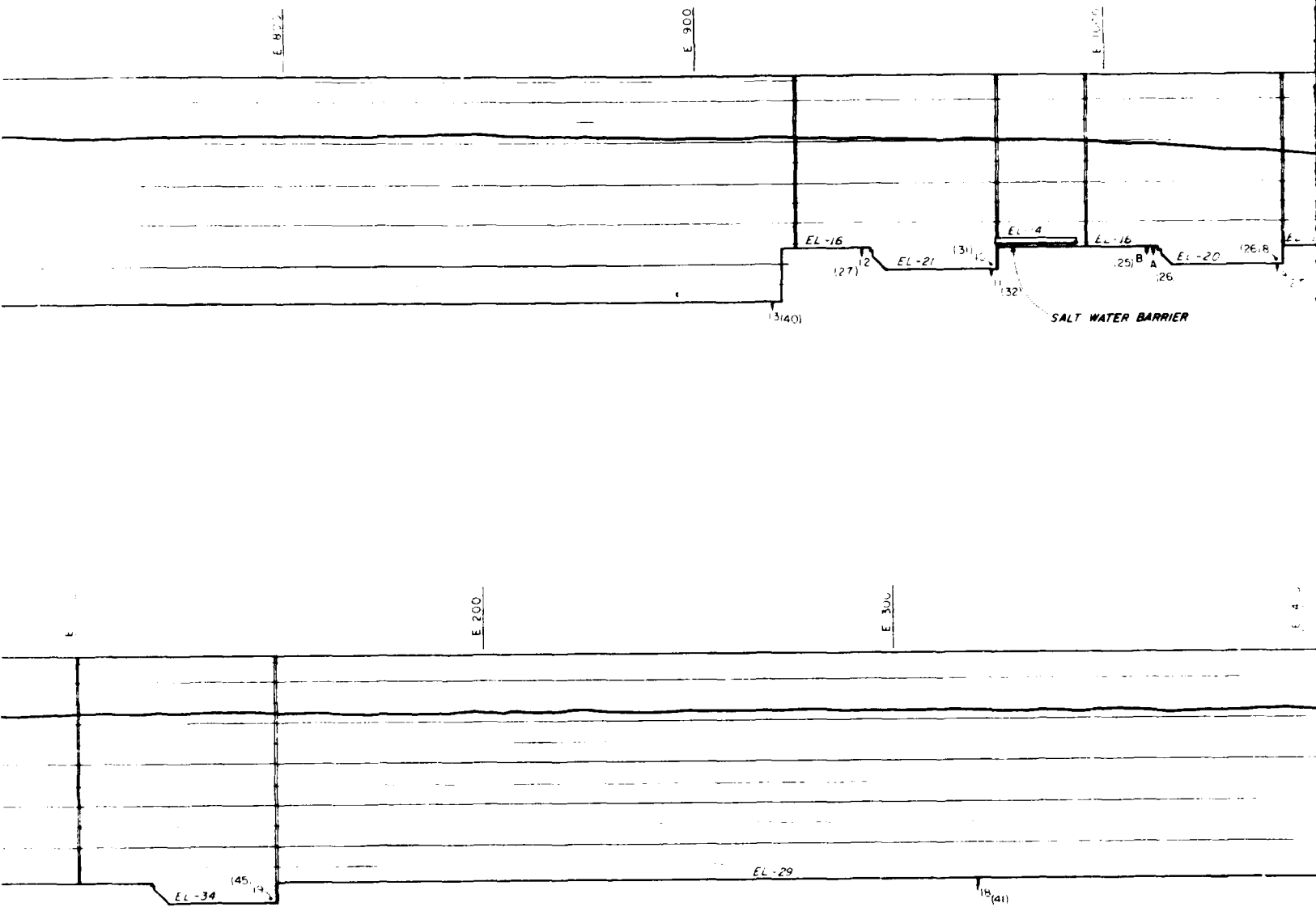
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LOCK DISCHARGE 48.700 CFS

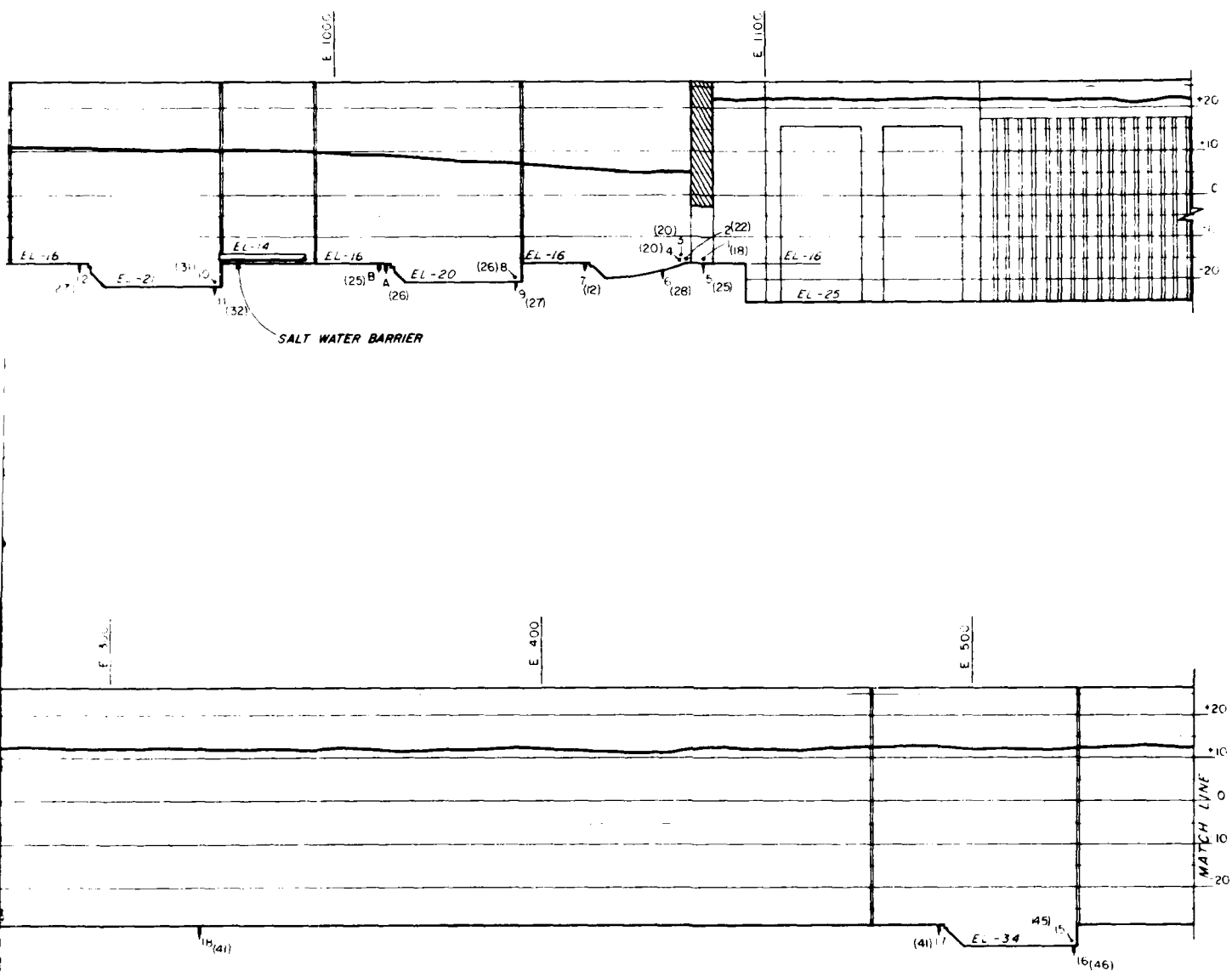
VELOCITIES

FOREBAY EL 22, TAILWATER EL+12
FREE FLOW



LEGEND

- WATER-SURFACE PROFILE ALONG RIGHT WALL
- (26) PRESSURE IN FEET OF WATER



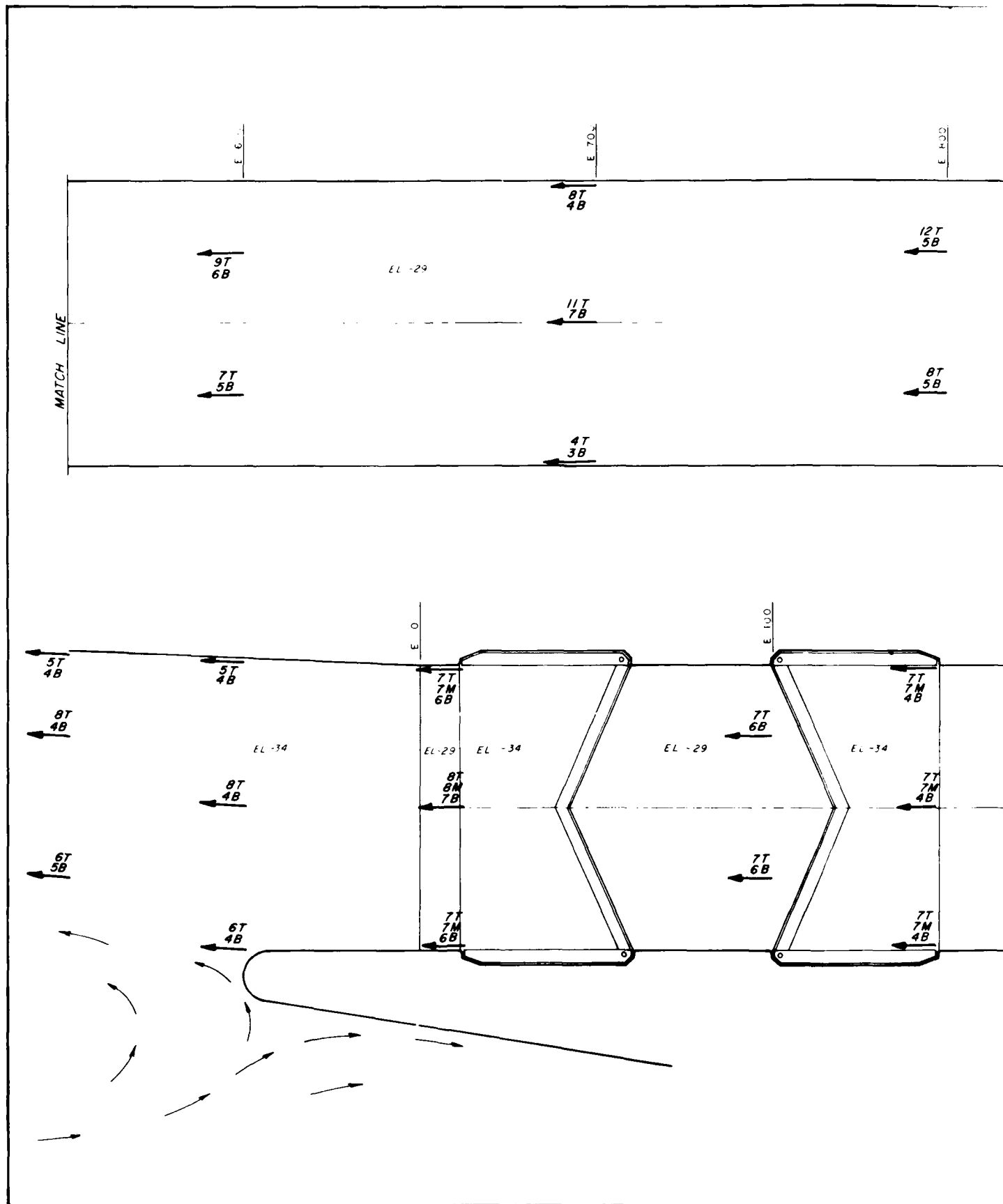
LOCK DISCHARGE 21 500 CFS

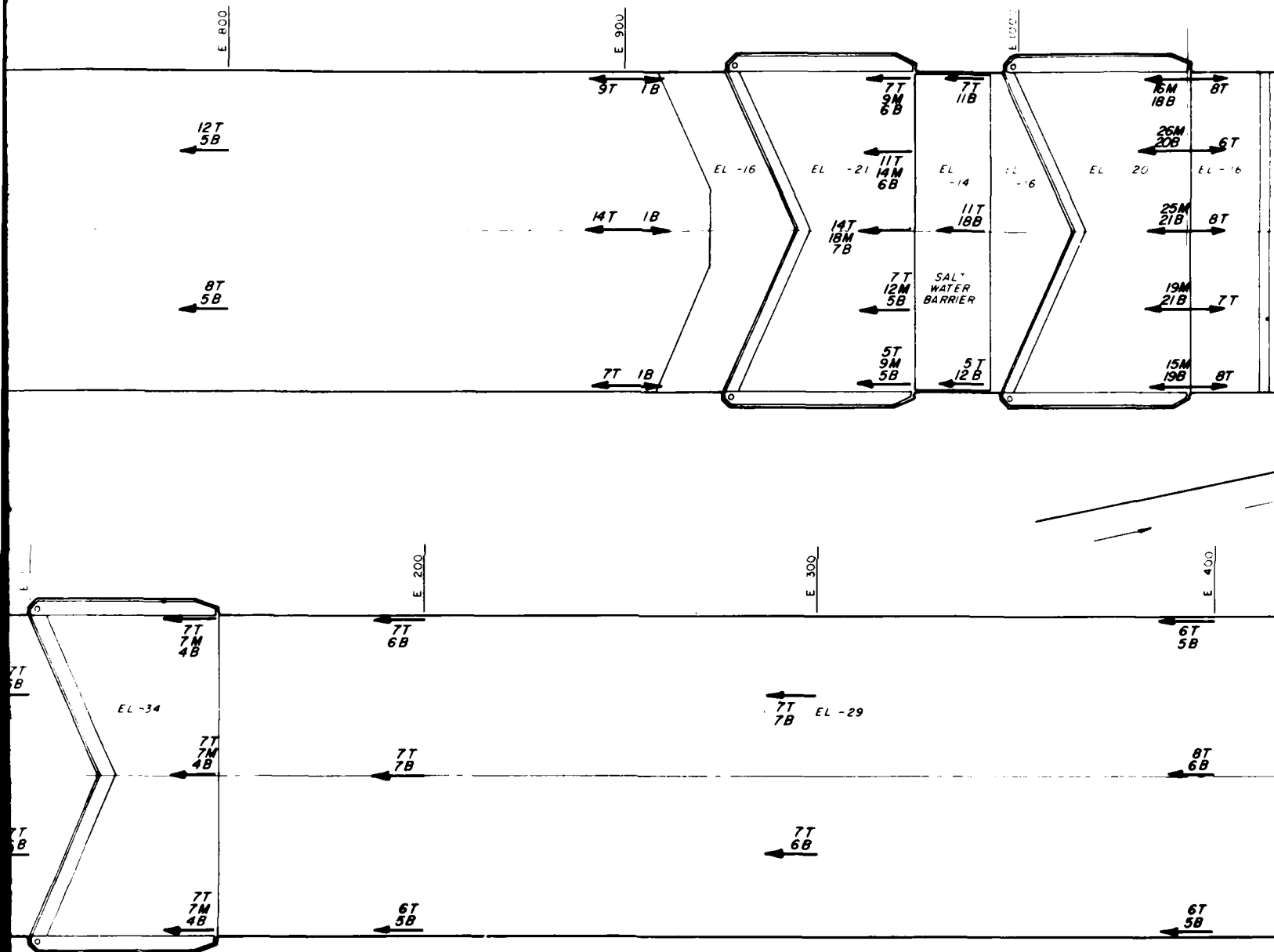
WATER SURFACE AND PRESSURES

FOREBAY EL 22; TAILWATER EL +12

BULKHEAD OPEN 13 FT

3

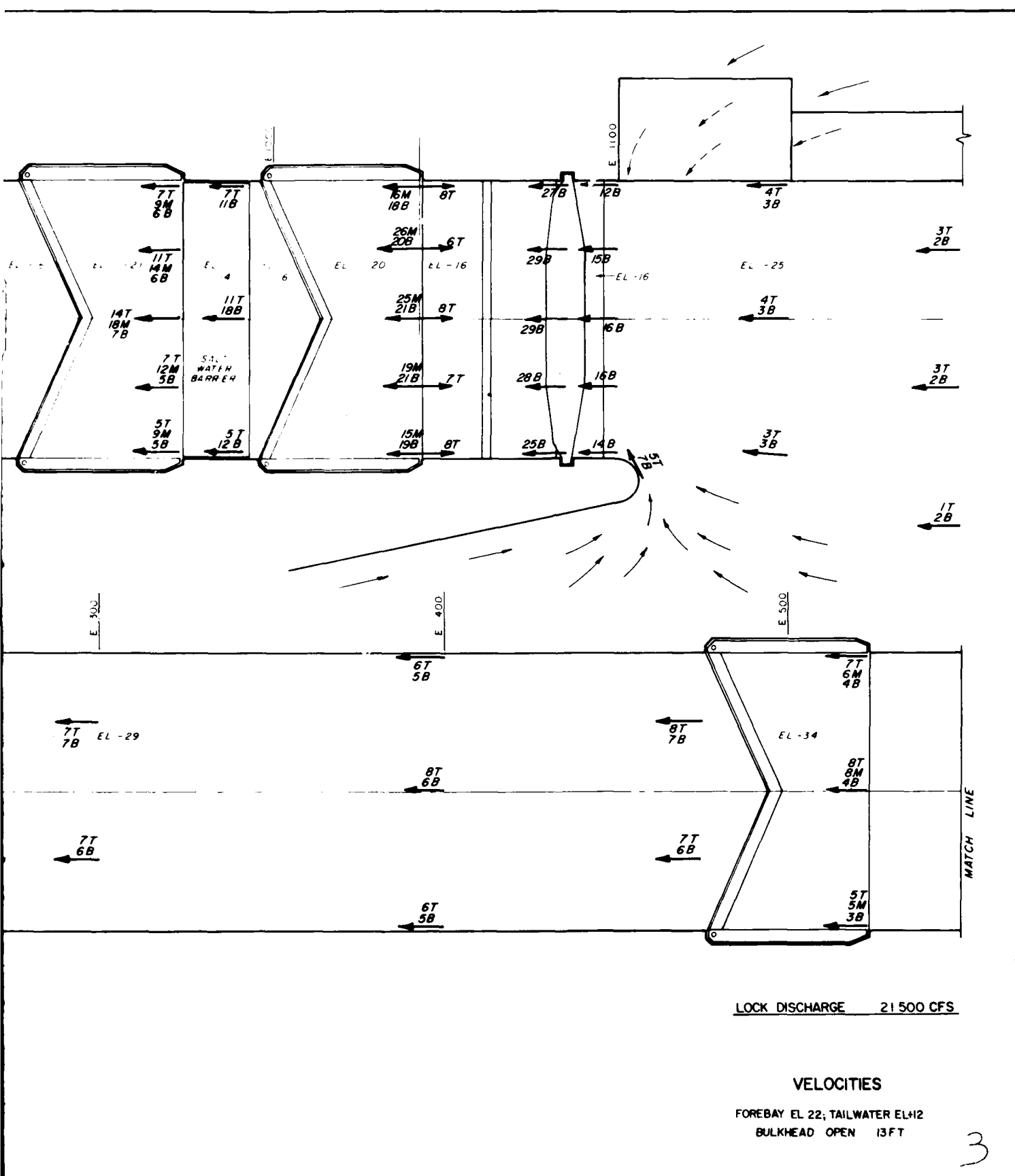


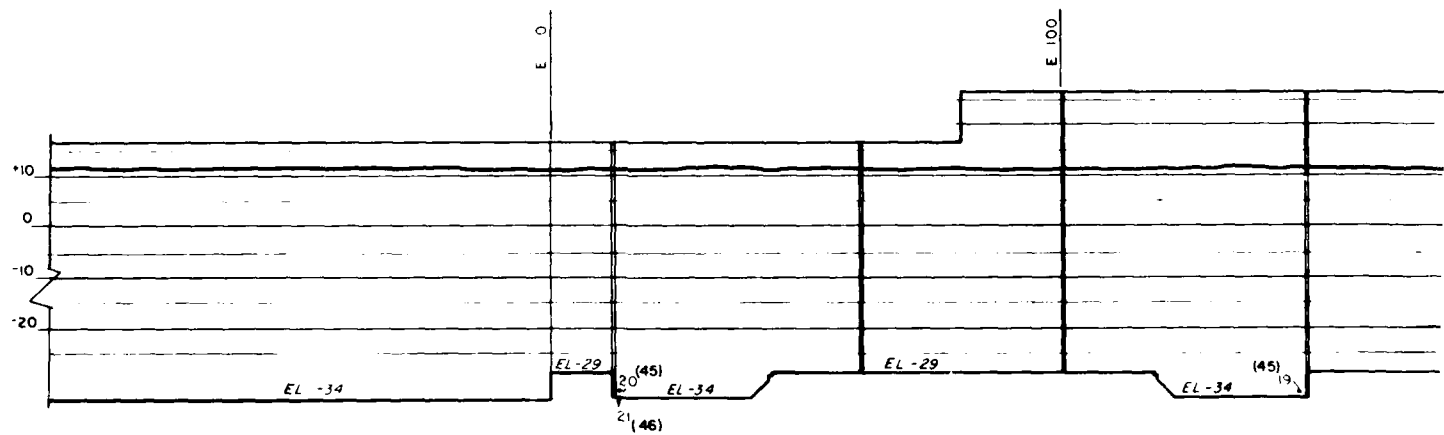
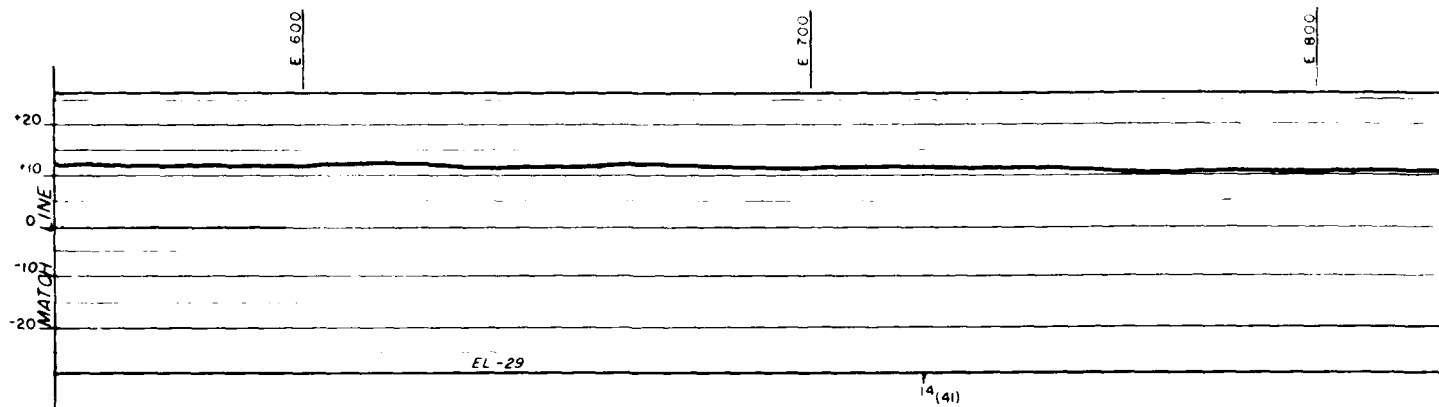


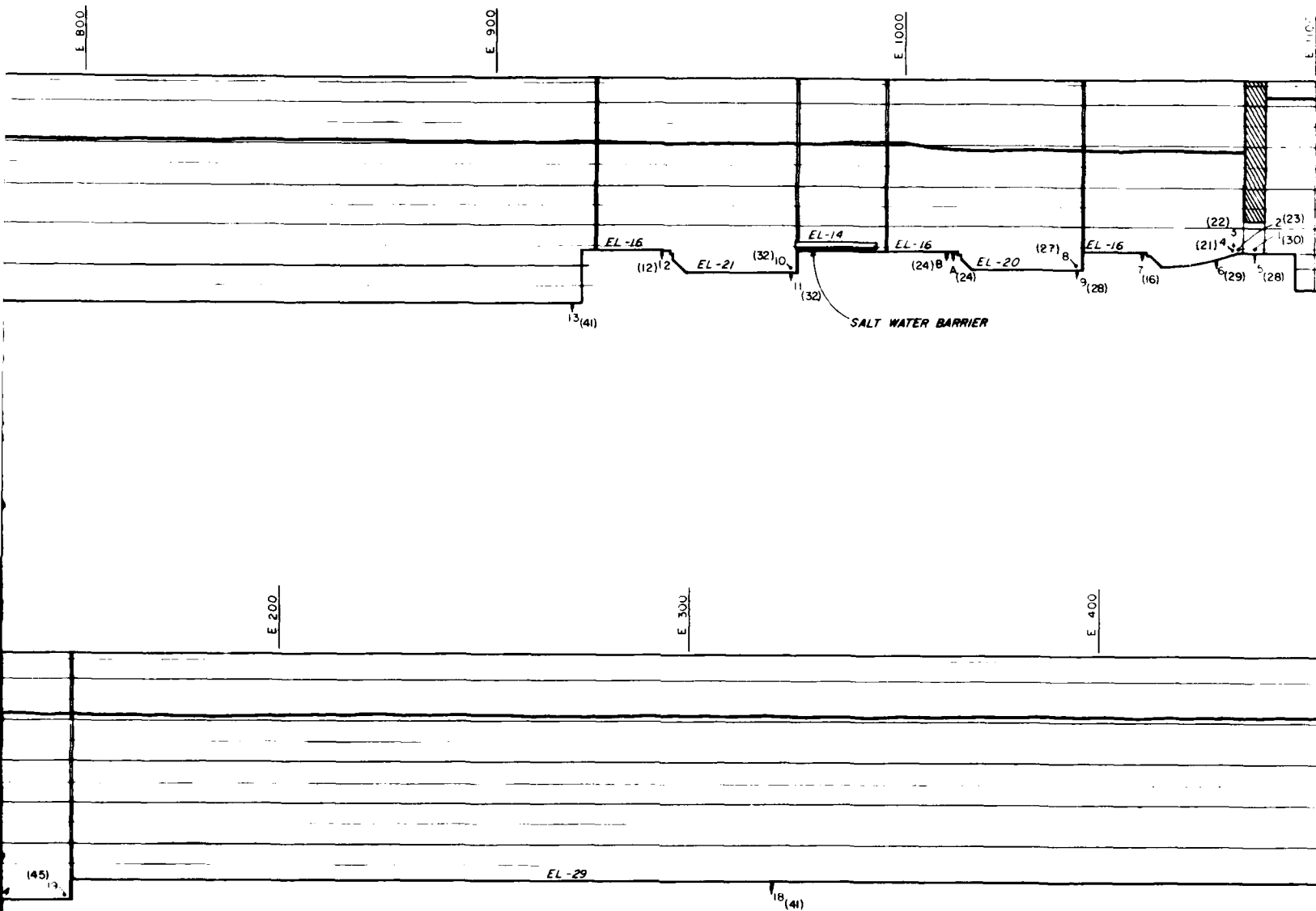
LEGEND

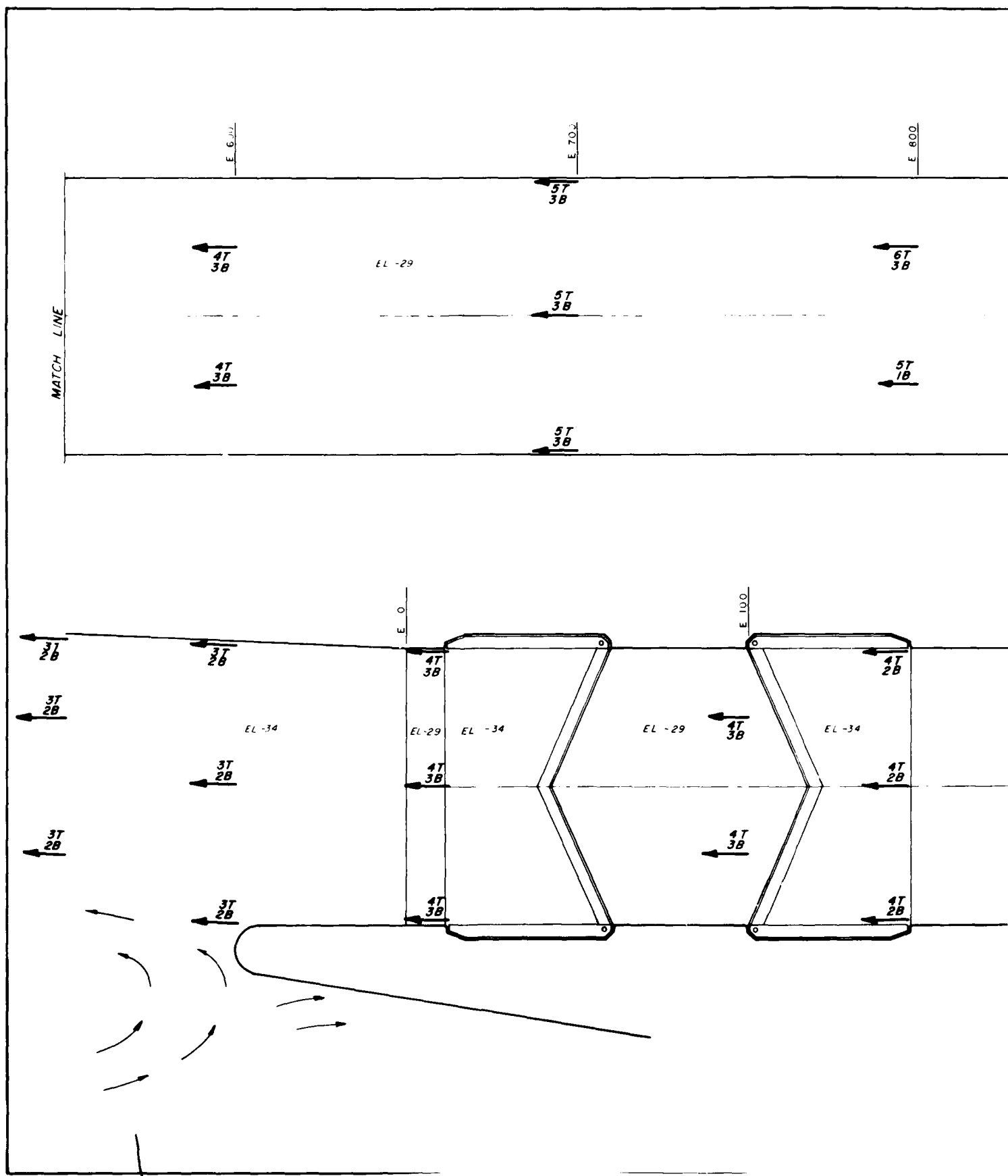
- ← VELOCITY IN FPS
- T 5-FT DEPTH
- M MID-DEPTH
- B 5 FT ABOVE BOTTOM

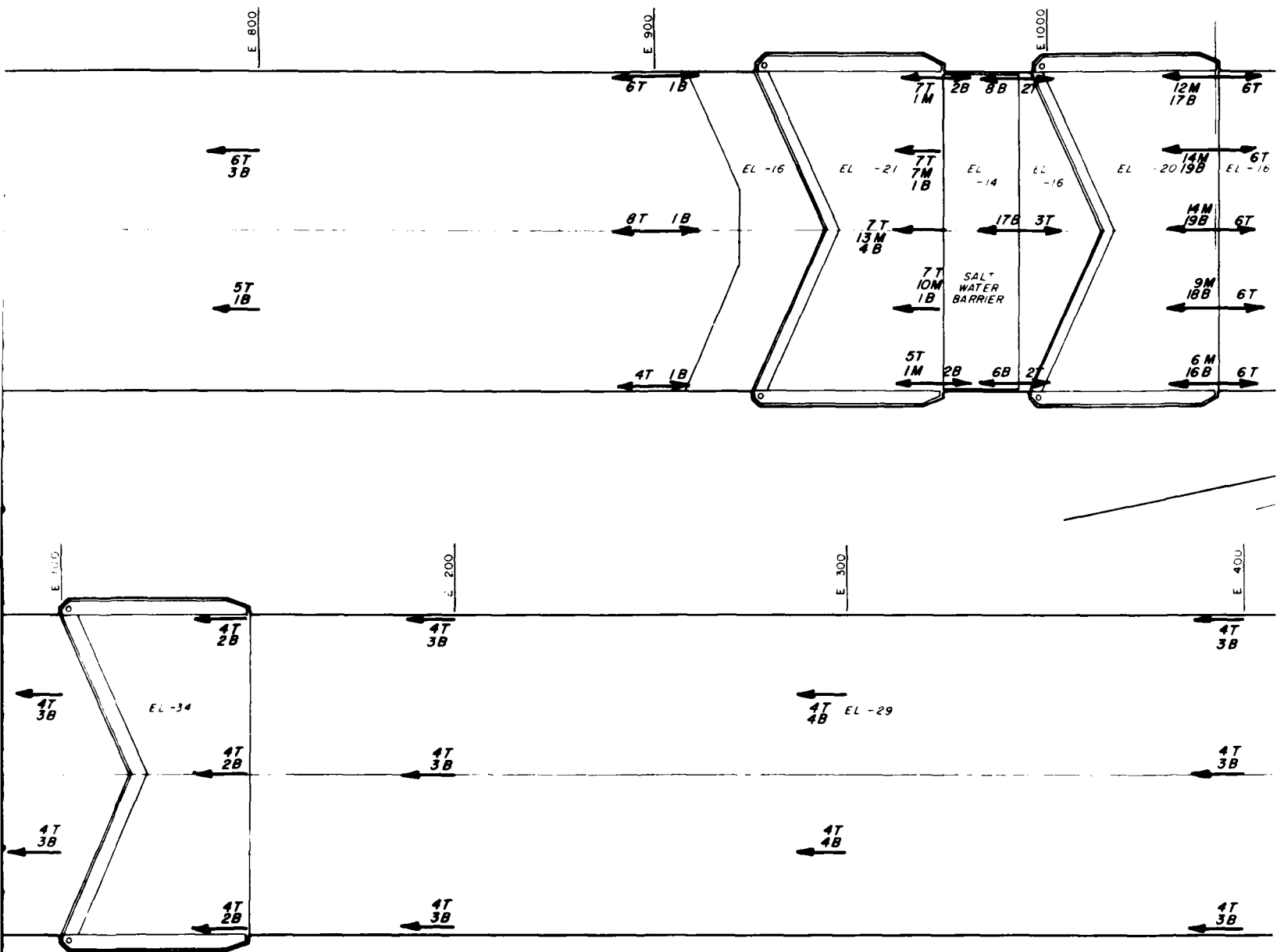
2







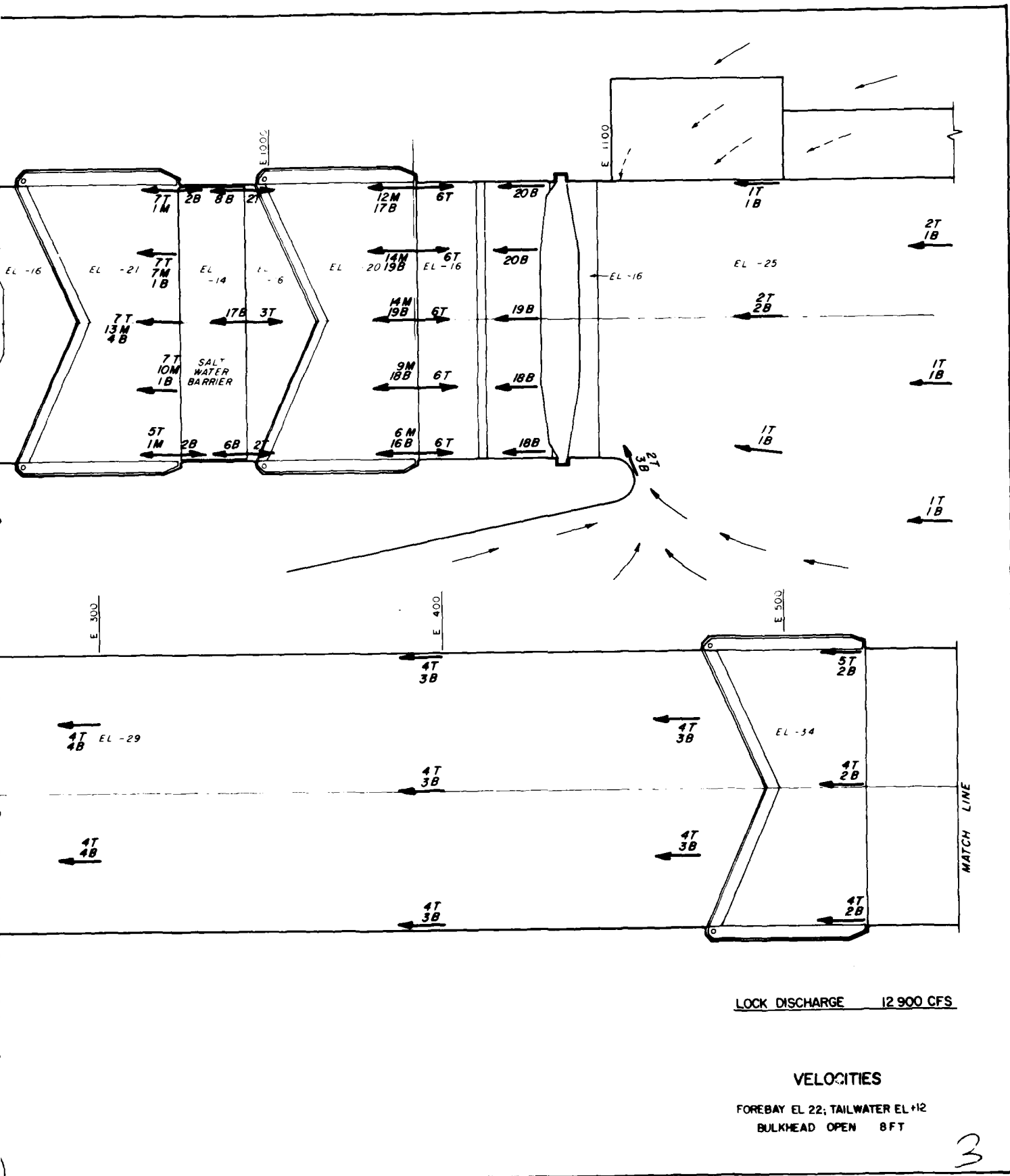


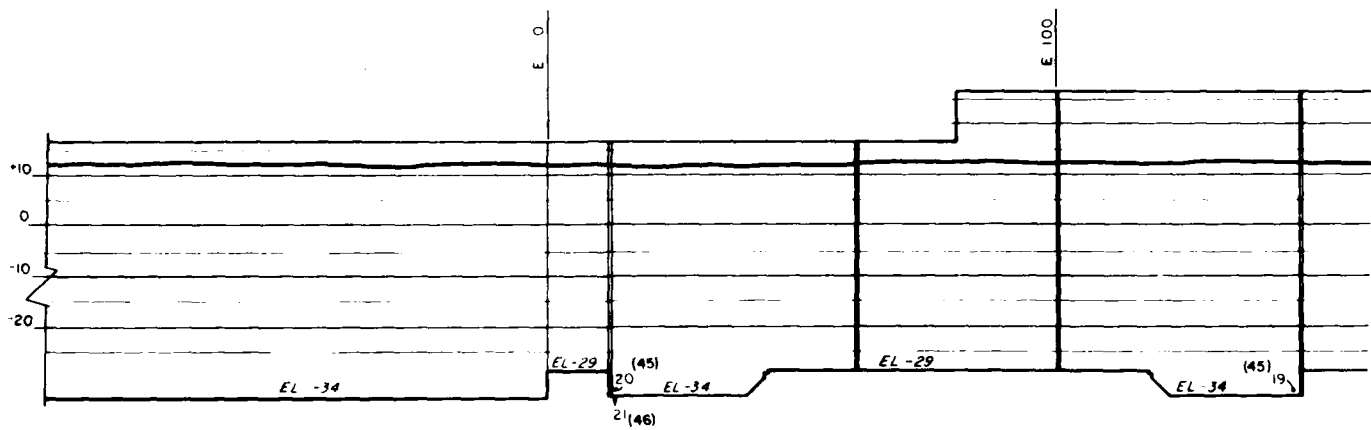
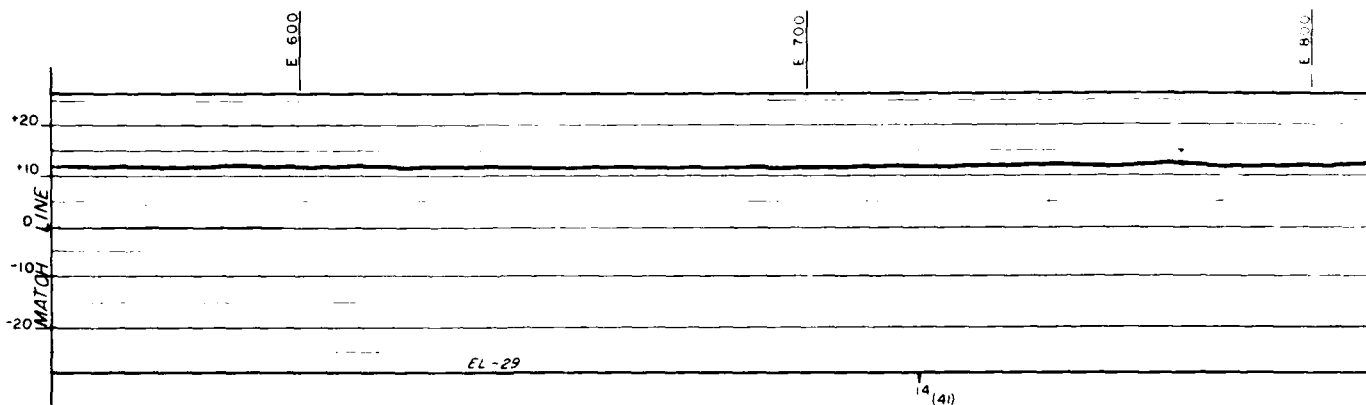


LEGEND

- ← VELOCITY IN FPS
- T 5-FT DEPTH
- M MID-DEPTH
- B 5 FT ABOVE BOTTOM

2





E 800

E 900

E 1000

E 200

E 300

E 400

EL-16

(28) 2

EL-21

(32) 10

EL-14

EL-16

(27) B

A

EL-20

(27)

(34) B

EL-16

(31) 9

7

12

13 (4)

11 (33)

SALT WATER BARRIER

EL-34

(45) 19

EL-29

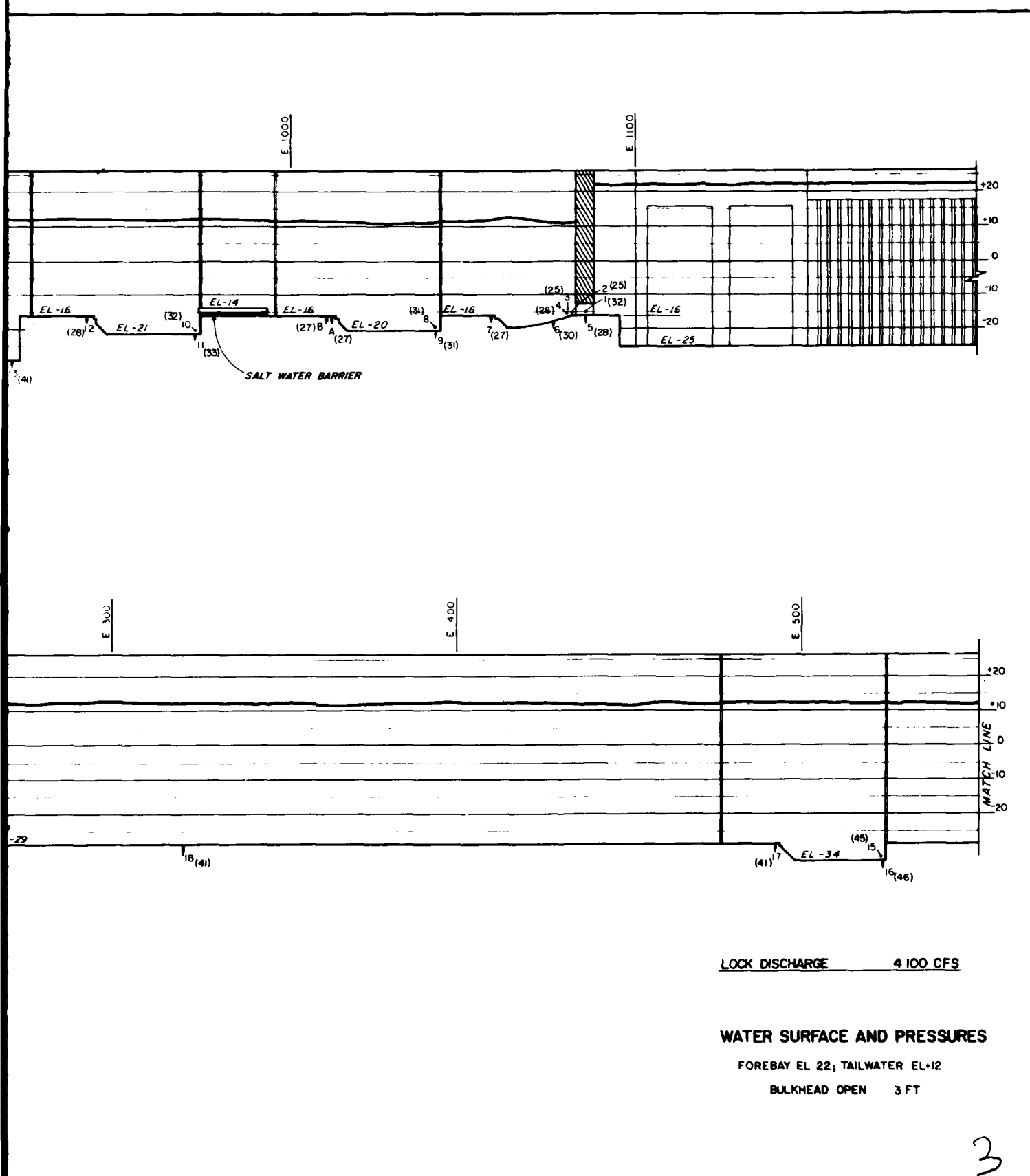
18 (4)

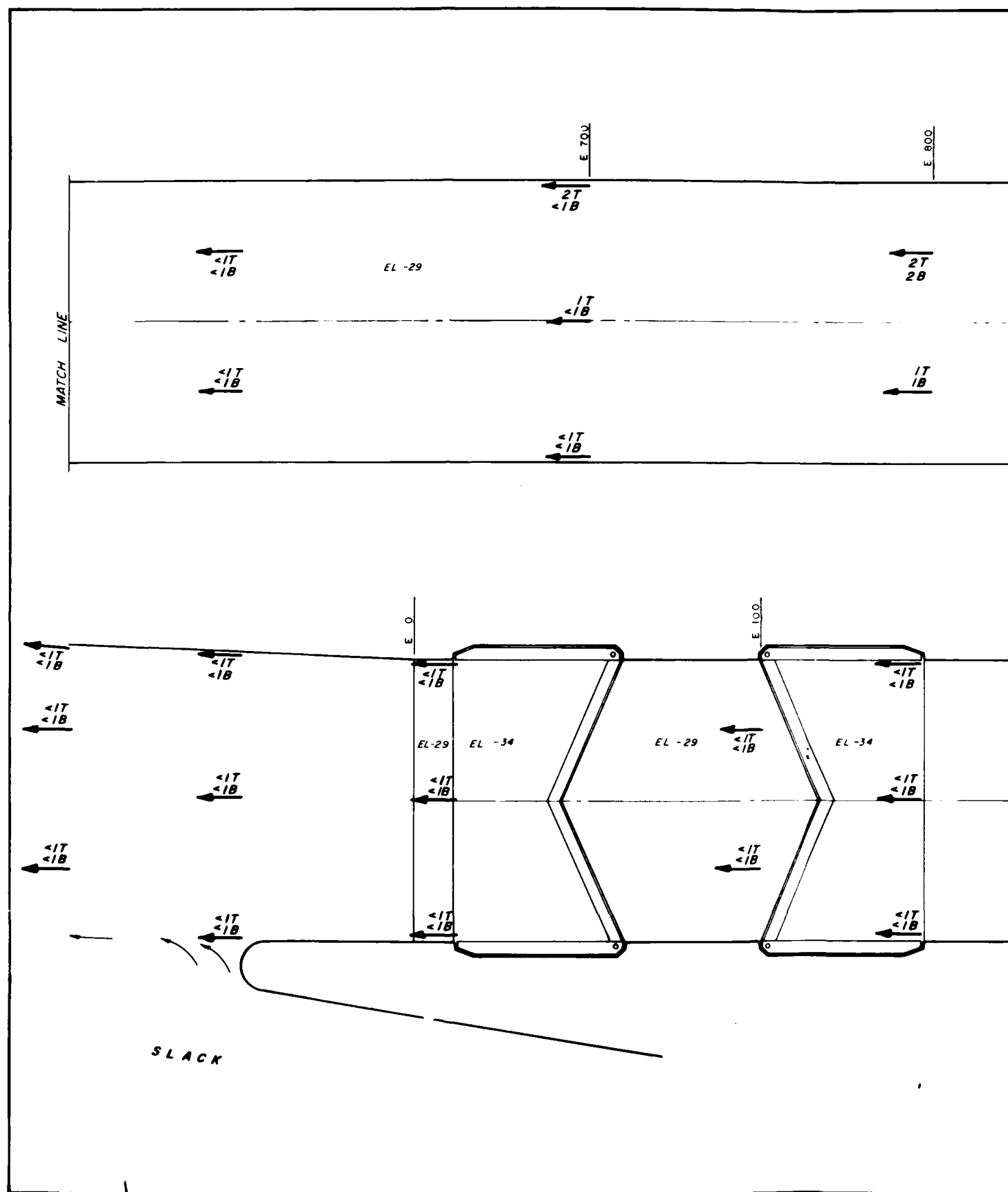
LEGEND

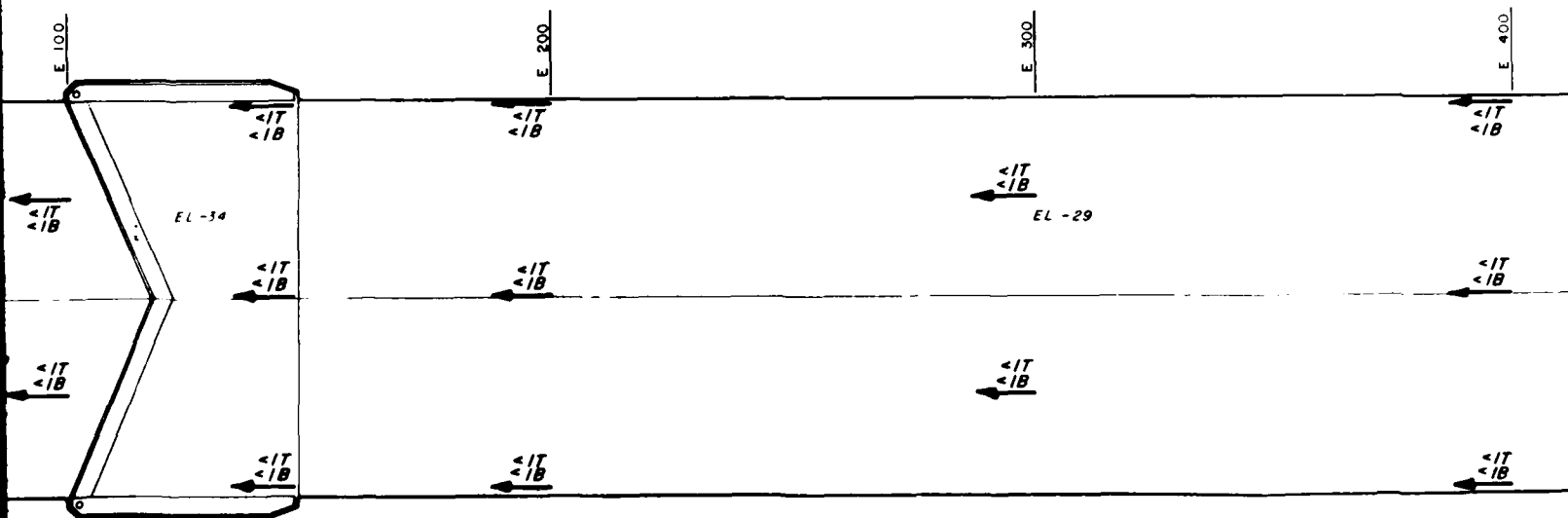
— WATER-SURFACE PROFILE ALONG
RIGHT WALL


(26) PRESSURE IN FEET OF WATER

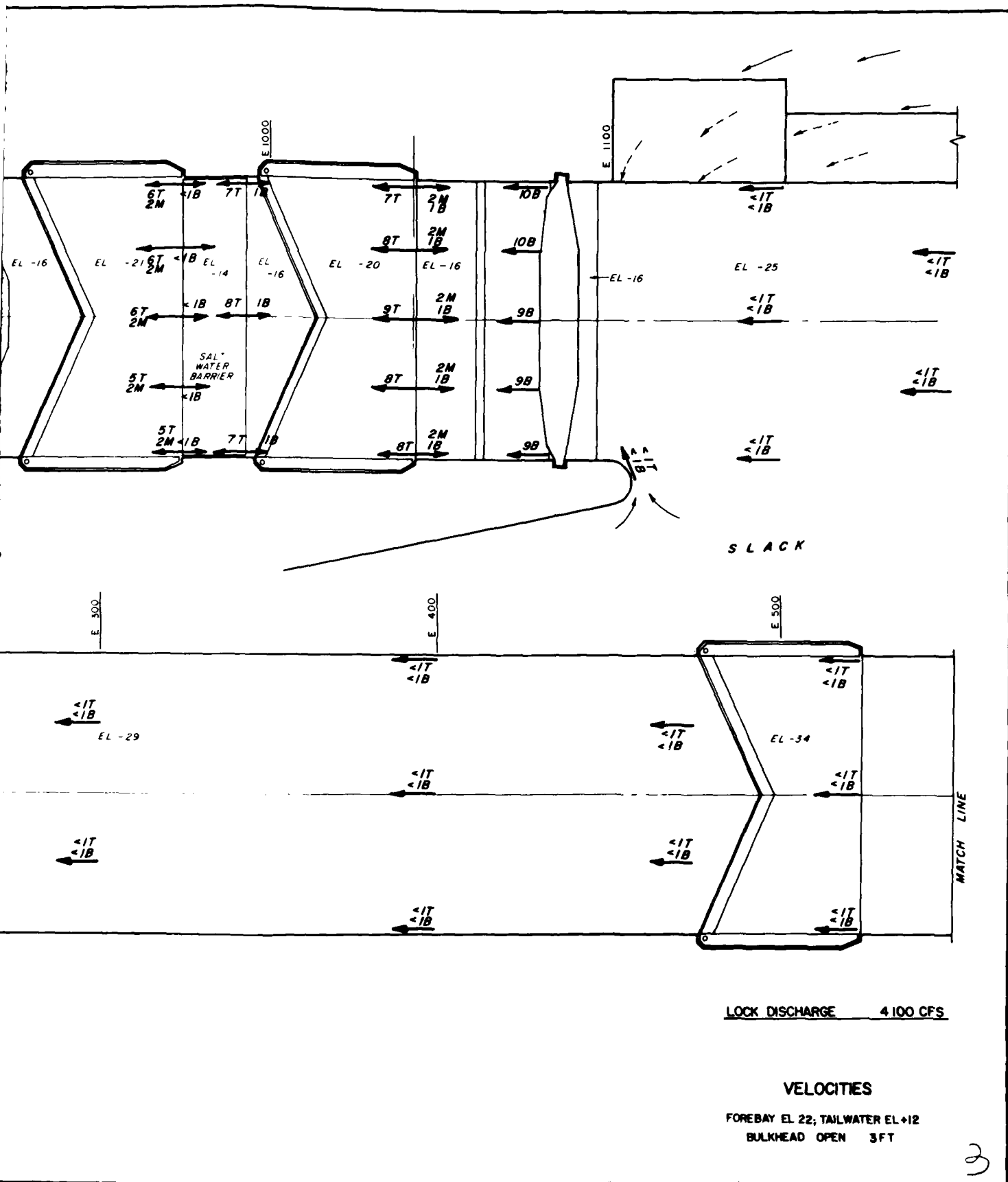
2

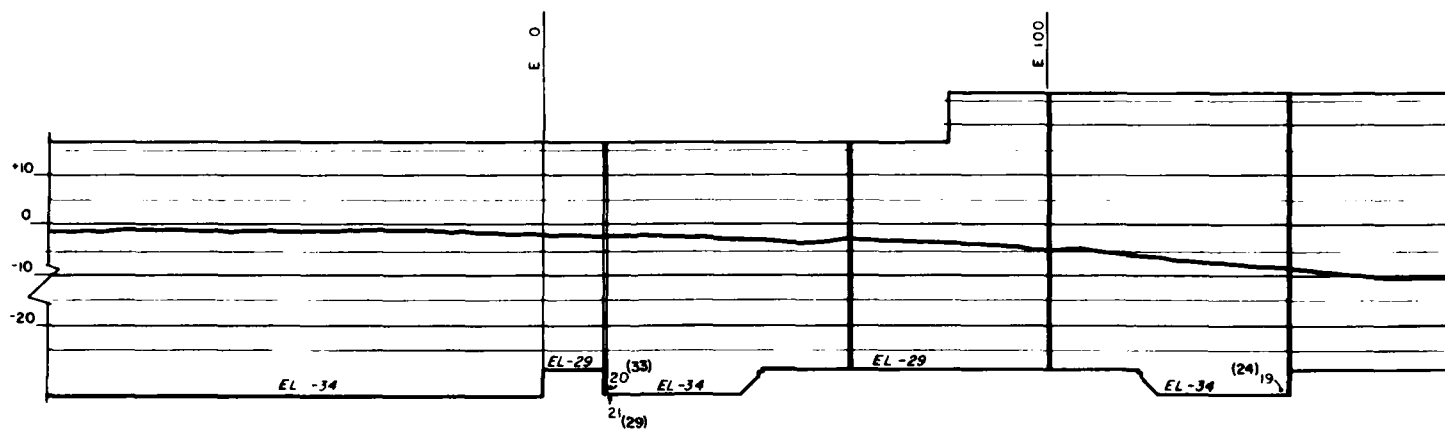
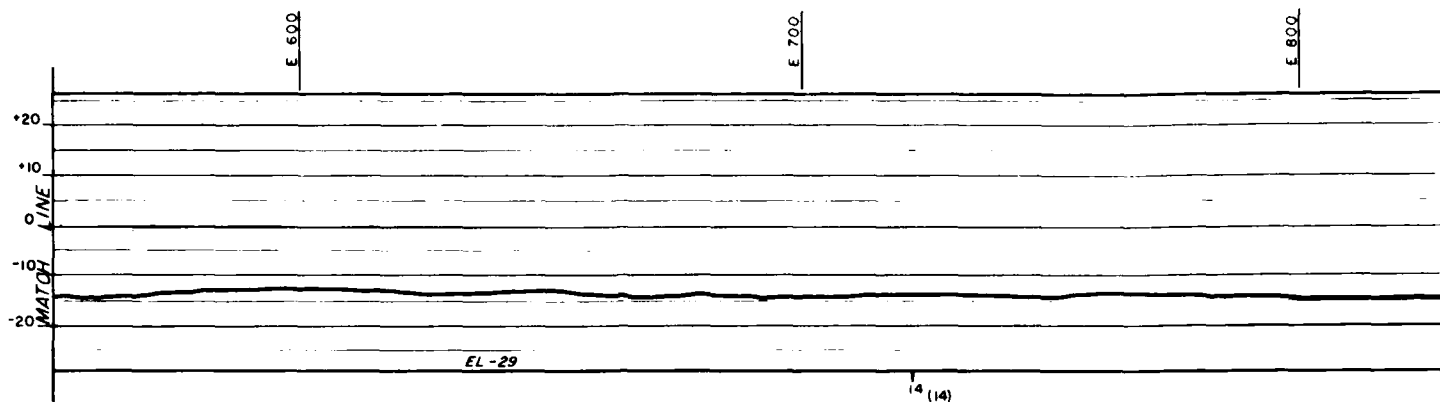


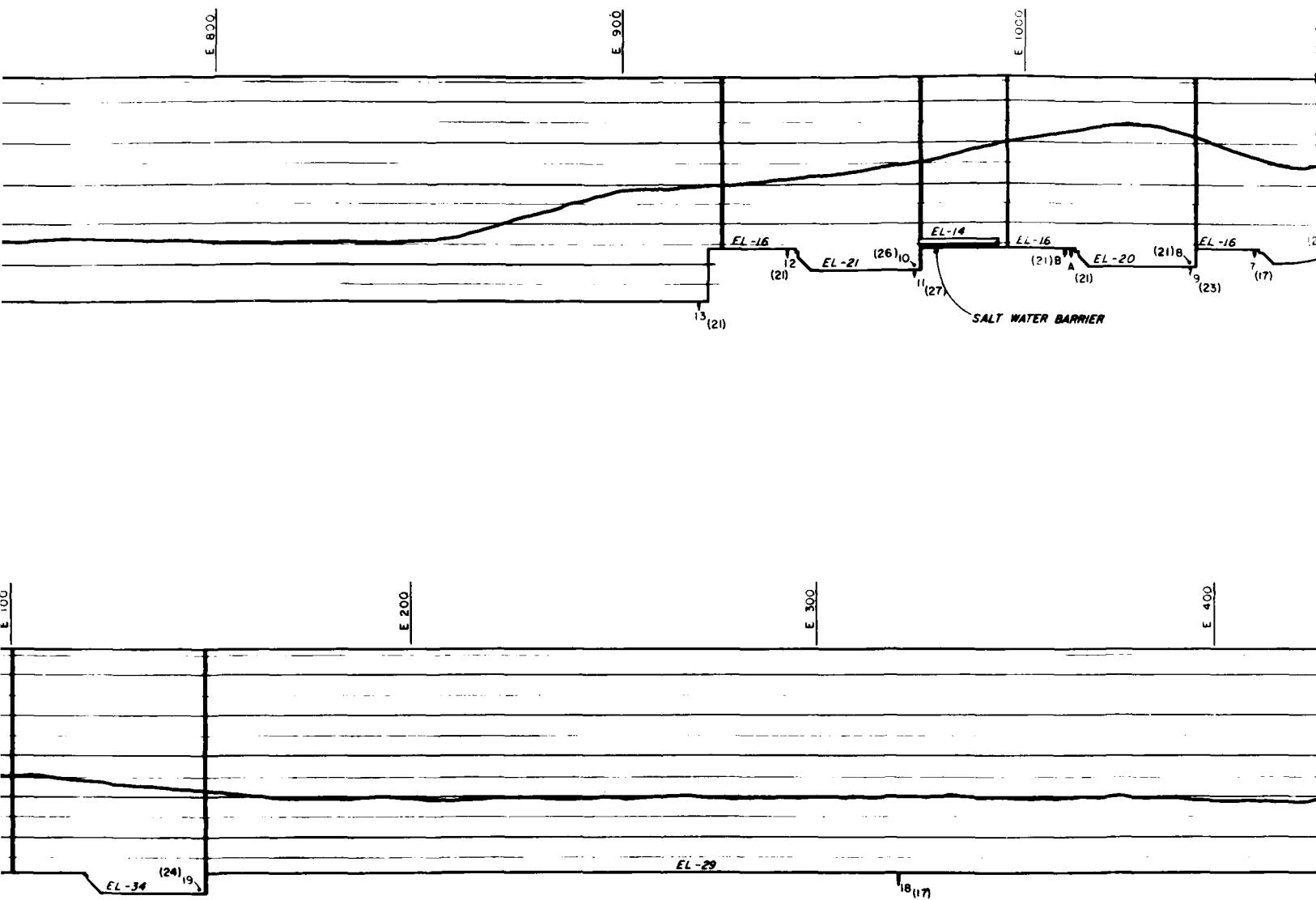




 VELOCITY IN FPS
 T 5-FT DEPTH
 M MID-DEPTH
 B 5 FT ABOVE BOTTOM



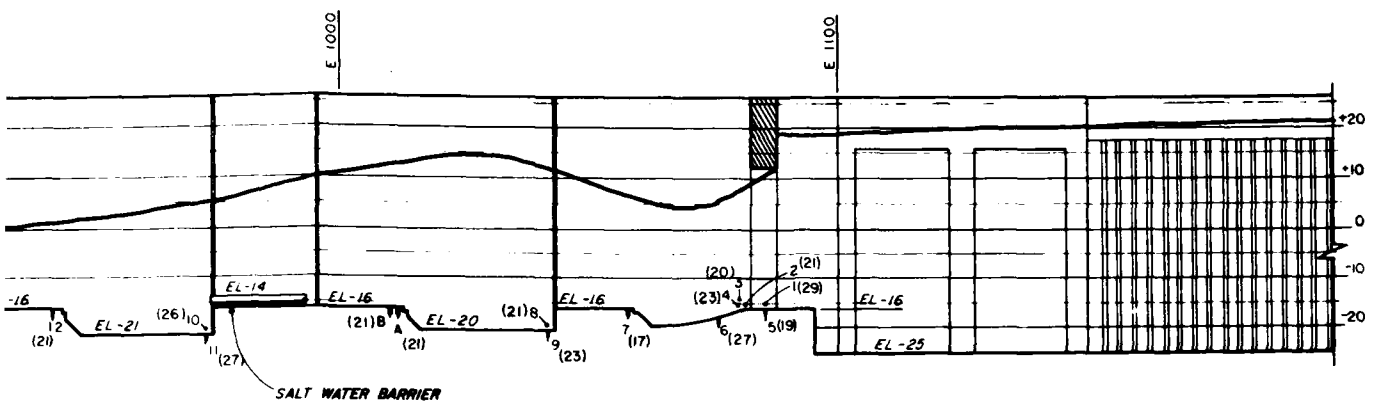




LEGEND

- WATER-SURFACE PROFILE ALONG RIGHT WALL
- (26) PRESSURE IN FEET OF WATER

2



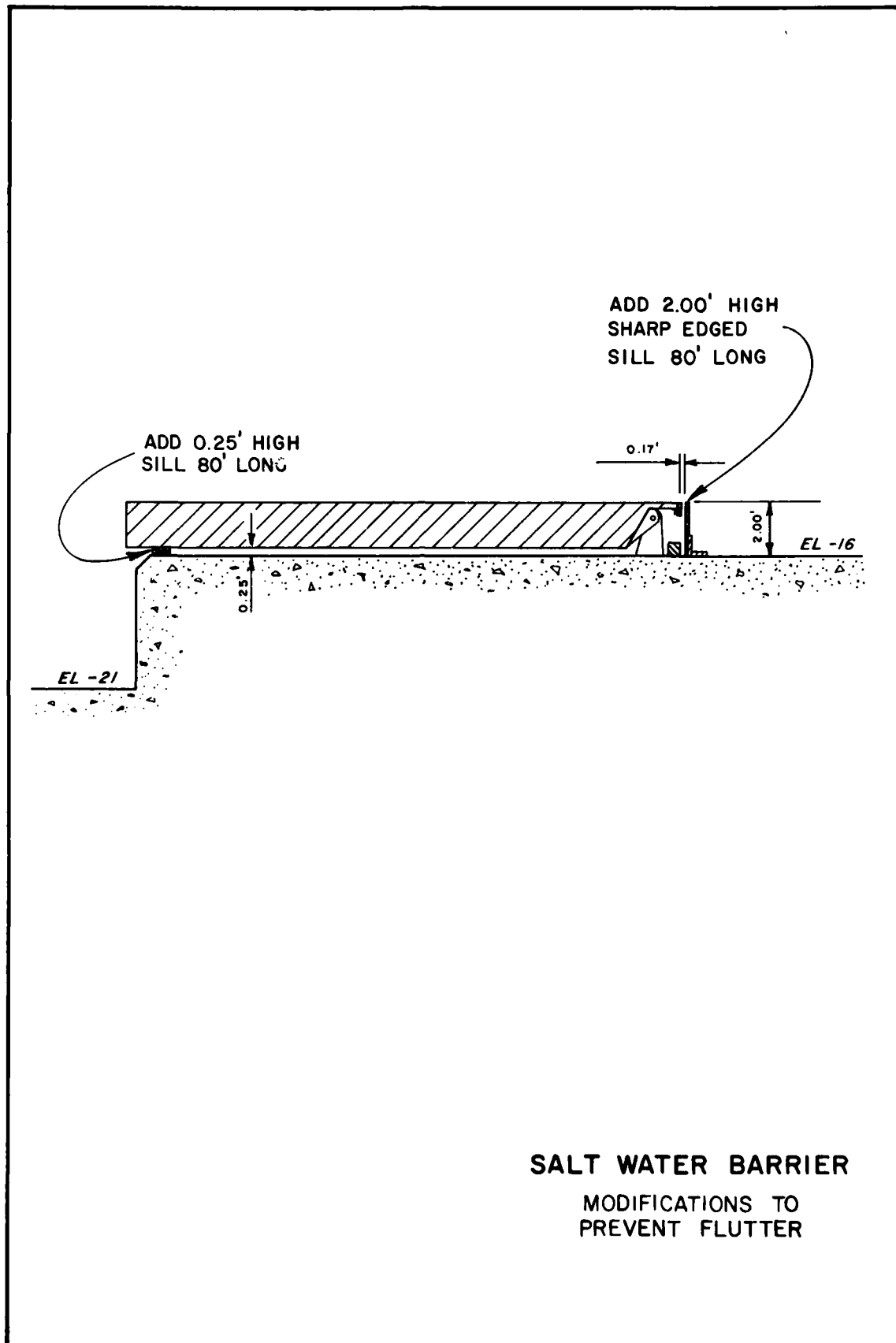
LOCK DISCHARGE 45 500 CFS

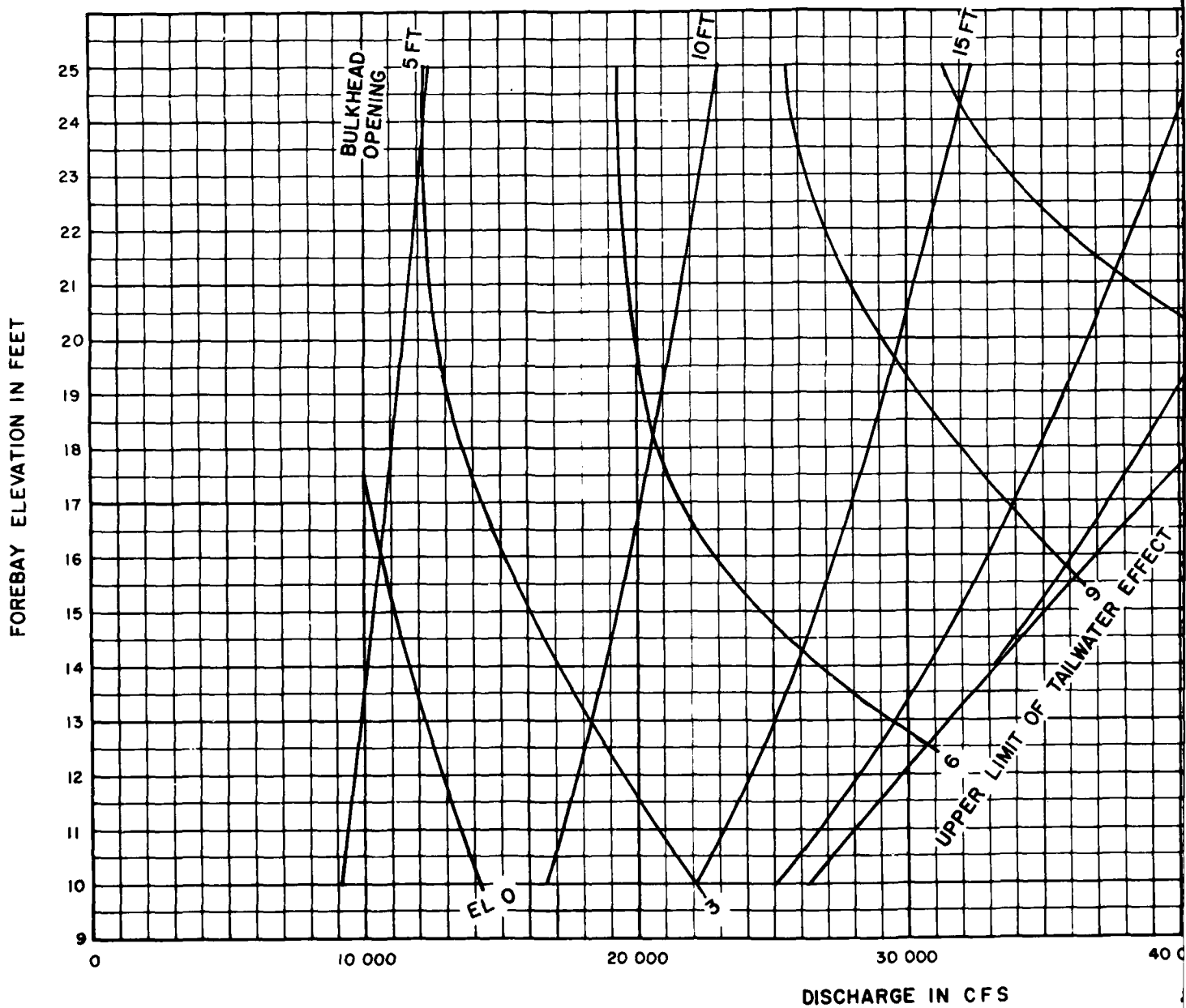
WATER SURFACE AND PRESSURES

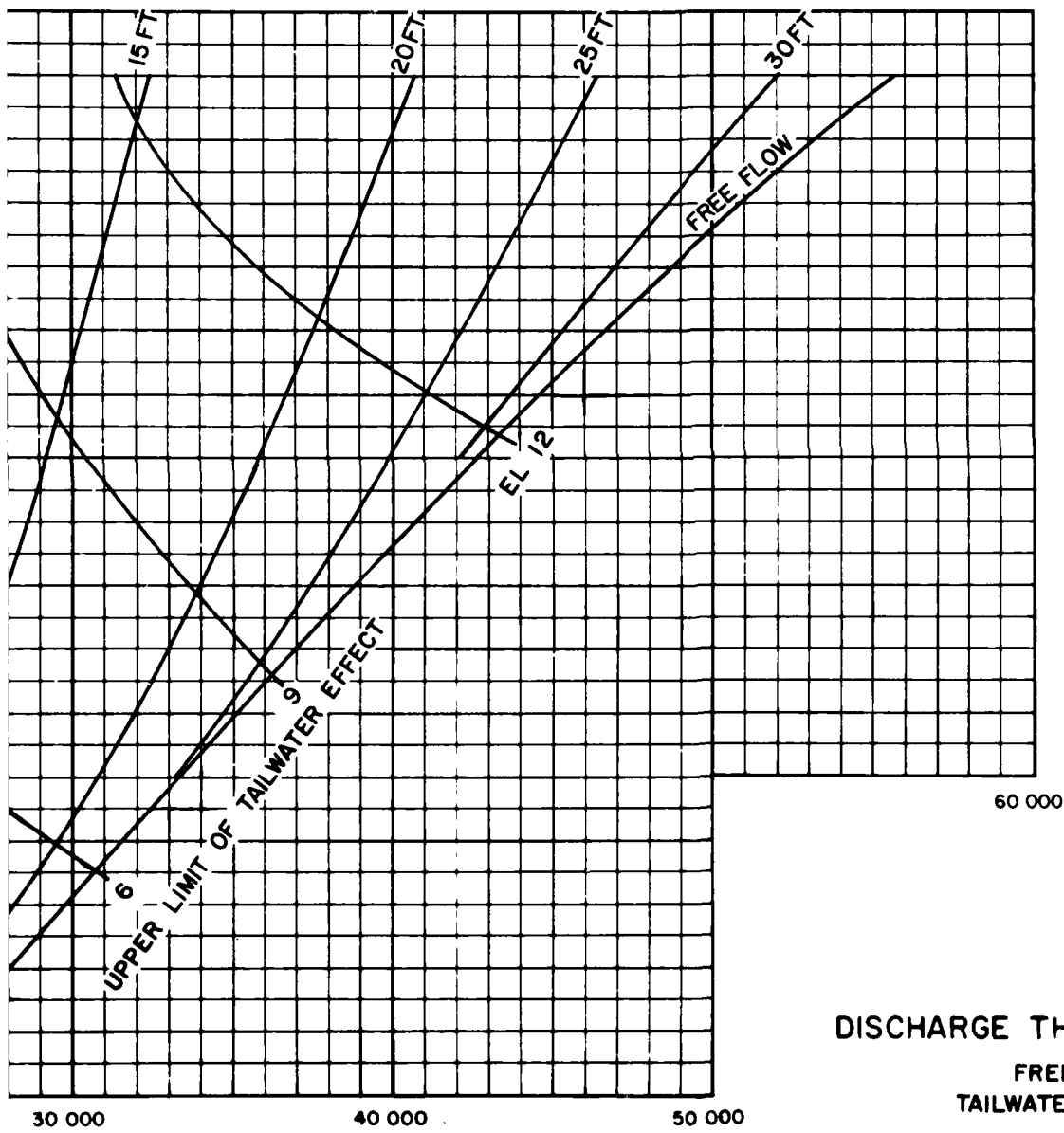
FOREBAY EL 22; TAILWATER EL -2

BULKHEAD OPEN 28 FT

3







DISCHARGE THROUGH LOCK
FREE OF
TAILWATER EFFECT

DISCHARGE IN CFS

2

